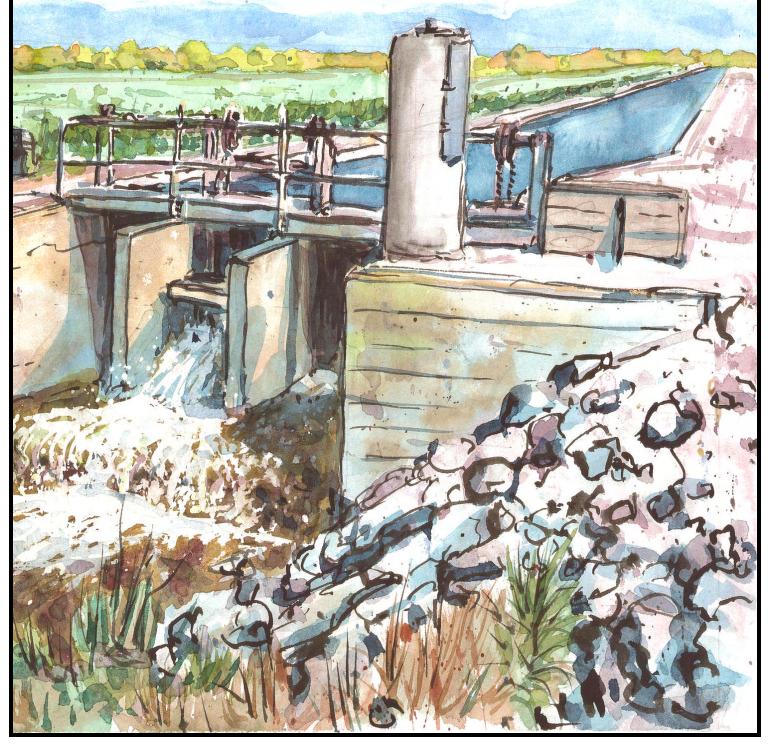


IRRIGATION CONSTRUCTION & ENGINEERING DESIGN STANDARDS August 2011



CONSTRUCTION & ENGINEERING STANDARDS

REVISION CHECK SHEET

REVISION	DATE	REVISION	DATE
1	09/24/96	16	08/22/11
2	08/20/97	17	
3	01/02/98	18	
4	07/08/98	19	
5	10/14/98	20	
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Dwg #	# OF Sheets	TITLE	DATE	REVISION
ES 101	I	CANAL AND DITCH DESIGN SPECIFICATIONS	01/27/92	Ι
ES 102	I	PERFORATED UNDERGROUND DRAINAGE LINES	10/21/00	С
ES 103	I	DESIGN STANDARD FOR SURFACE DRAINS	04/00/00	В
ES 104	I	FLOW OF WATER IN CONCRETE PIPE	09/12/86	
ES 105	I	FLOW OF WATER IN PLASTIC PIPE	09/12/86	
ES 106	I	FLOW THROUGH GATES FREE AND SUBMERGED DISCHARGE	09/12/86	
ES 107	I	FLOW OVER SHARP CRESTED WEIR	09/12/86	
ES 108	I	DESIGN STANDARD FOR RETAINING WALLS AND WEIR OVERPOURS	09/12/86	
ES 109	L	DESIGN STANDARD FOR CANAL WALKWAY	09/12/86	
ES IIO	2	UNDERGROUND FACILITIES CROSSING DISTRICT IRRIGATION FACILITIES	01/09/97	В
ES III	L	REQUIRED EASEMENTS AND RIGHTS-OF-WAY	05/11/99	В
ES II2	12	IRRIGATION PUMP AND WELL STANDARDS	03/21/89	
ES II3	I	CONDITIONS FOR DOWNSIZING IRRIGATION FACILITIES	03/23/92	Ι
ES 300	I	IRRIGATION SYSTEM WATER QUALITY	11/00/99	А

INDEX ENGINEERING STANDARDS	WATER & POW	IRRIGATIO ENGINEERII STANDARD	NG
	SHEET		
	1 OF 1	dwg no. ES 1 G	REVISED 09/20/2011

CANAL AND DITCH DESIGN SPECIFICATIONS

VERTICAL TRANSITIONS IN GRADE Horizontal (H) : Vertical (V)

Main Canals and Laterals: 4:1 OR 15° MAXIMUM

Improvement District and Private Ditches: 2:1 OR 26° MAXIMUM

HORIZONTAL TRANSITION IN ALIGNMENT FROM CANAL LINING TO A STRUCTURE:

1 TRANSVERSE : 2 LONGITUDINAL (Minimum)

STANDARD DESIGN FREEBOARD

	Inche	s Feet
Upper Main Canals	18"	1.5'
Main Canals (800 - 2050 cfs)	12"	1.0'
Laterals (15 - 250 cfs)	6"	0.5'
Ditches (5-30 cfs)	4"	0.2'

MANNING'S "N" FOR CONCRETE LINED CANALS:

0.015

MAXIMUM DESIGN HEADLOSS TOTAL FOR CANAL SIPHONS: Up to 100 feet in length including transitions: 0.2 FT. Over 100: A site specific engineering design is required.

FOR DESIGN OF CANAL SIPHONS FOR ROAD CROSSINGS: (To include transition structures) Refer to the Current Cal-Trans Siphon Standards.

BANKS OF DITCHES: Refer To CS 143

BANKS OF CANALS: Refer to CS 127

	TURLOCK IRRIGA	TIO			TEM ADMINISTRATION								
				-	DITCH DESIGN								
1	Revised Wording for Clarification	HBB				BDH	1-27-92						
	Initial Ieres	JAS	RRV			BDH	9-12-86	SHEET DWG		ES 101			
REV	DESCRIPTION	INIT	СНК	RV*D	RVD	APP	DATE	1 OF 1	NO.	ES 101			

PERFORATED UNDERGROUND DRAINAGE LINES

AVERAGE DESIGN FLOW: 0.03 gpm/lf of line

LATERAL SPACING: Shall not exceed 500 ft.

MATERIAL: PERFORATED POLYETHYLENE (ASTM F 405, F 667).

All slots and holes shall be free of tag ends and other material.

GRAVEL ENVELOPE:

The envelope material shall be washed gravel, free of organic matter, clays, and other deleterious substances that could, in time, change the hydraulic conductivity of the envelope. Envelope material is to be well graded gravel with 100% passing .75" clear square screen openings, no more than 30% passing a #60, and no more than 5% passing a #200 US standard series sieve. The envelope shall be placed completely around the pipe with a minimum thickness of 3 inches.

MINIMUM DEPTH FOR DRAIN LINES: 5.5'

<u>SIZE</u>	FLOW (cfs)	FLOW (gpm)
6"	0.15	67
8"	0.33	148
10"	0.61	274
12"	0.96	431
15"	1.70	763

LINE CAPACITY CHART SLOPE: 1.0'/1000' (0.001) MANNINGS "n"=0.015

TRENCH GRADE:

The minimum grade except where otherwise specified by the engineer is 1 foot in 1000 feet. The maximum allowable gradual departure from grade is 0.1 feet. No uphill grades are allowed.

LAYING THE DRAIN TUBING:

The plastic tubing shall be well-bedded with the envelope material completely surrounding the tubing. The maximum allowable stretch of the tubing is 5%. Care shall be taken to prevent damage to the tubing from rock and clods during the back-filling operation.

END RETURN/VENT:

A capped end return to one foot above natural ground shall be provided at the start of each drain line to act as an air vent.

= //	TURLOCK IRR	CONSTR	IRRIGATION UCTION STANDARD)S								
C STDS. COM. APPROVAL SER JUE STB								<u></u>				
С	Revised Mannings "n"						10-21-00	PERFORATED UNDERGROUND				
в	Revised standards — include const. spec.	BDH			BDH	BLL	9-3-96					
Α	Revised wording for clarification	HBB				BDH	1-27-96	DRAINAGE LINES				
	INITIAL ISSUE	JAS	RRV			BDH	9–12–86	SHEET	ES 102C	1		
REV	DESCRIPTION	INIT	СНК	RV℃D	APP	APP	DATE	1 OF 1	ES IUZC	L PAGE		

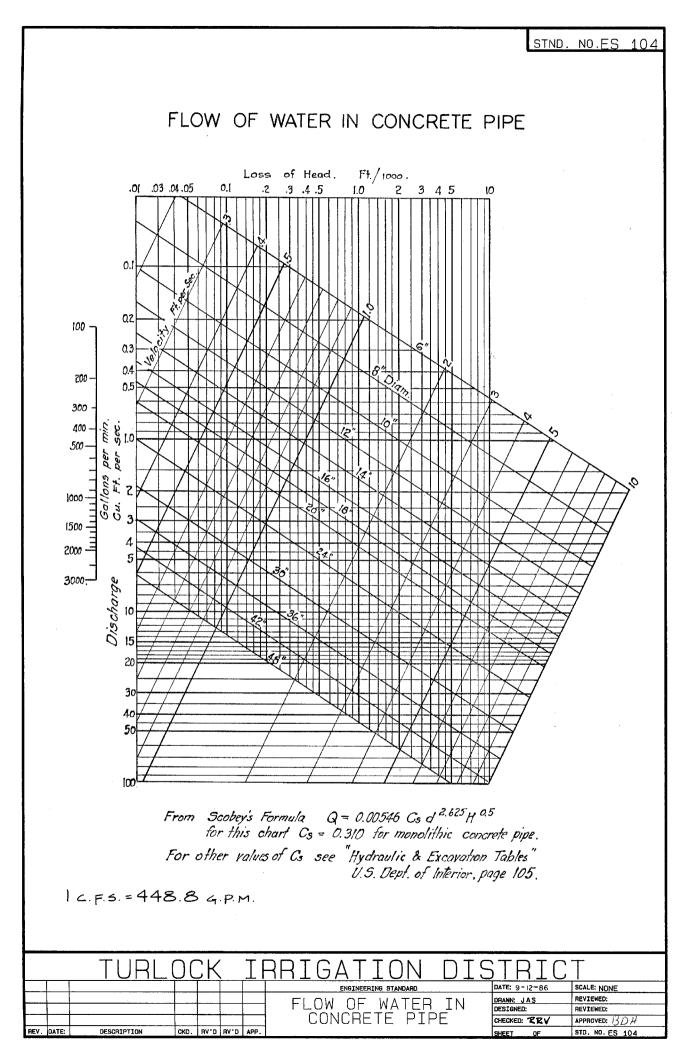
DESIGN STANDARD FOR SURFACE DRAINS

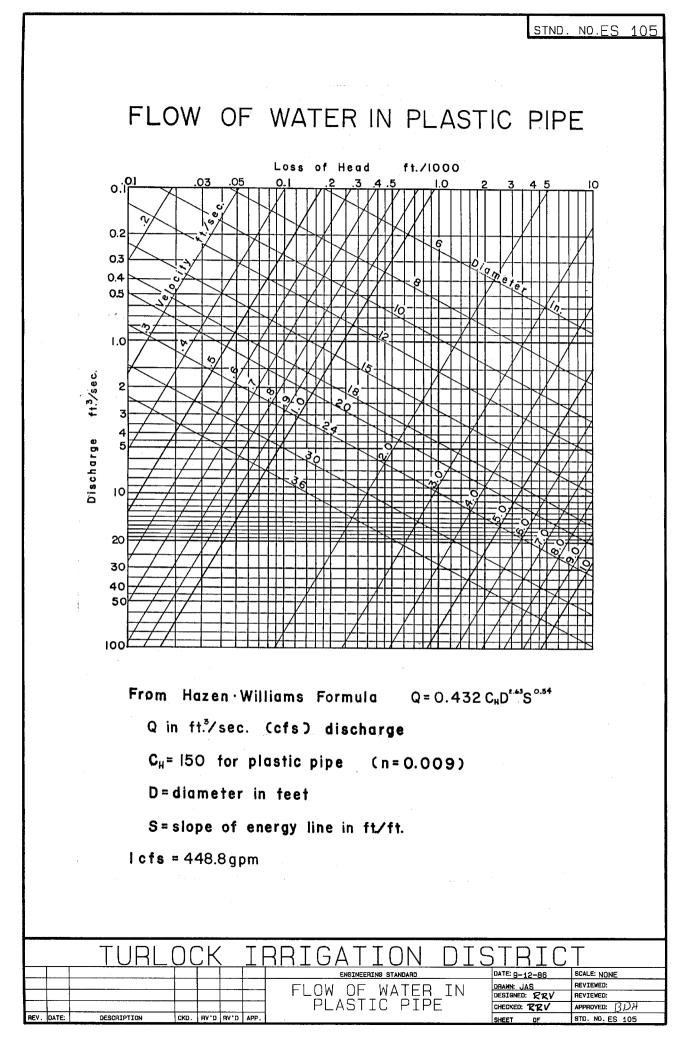
A STANDARD OF 5 CFS PER 1000 ACRES SHALL BE USED AS THE DESIGN FLOW FOR THE PIPING OF OPEN SURFACE DRAINS

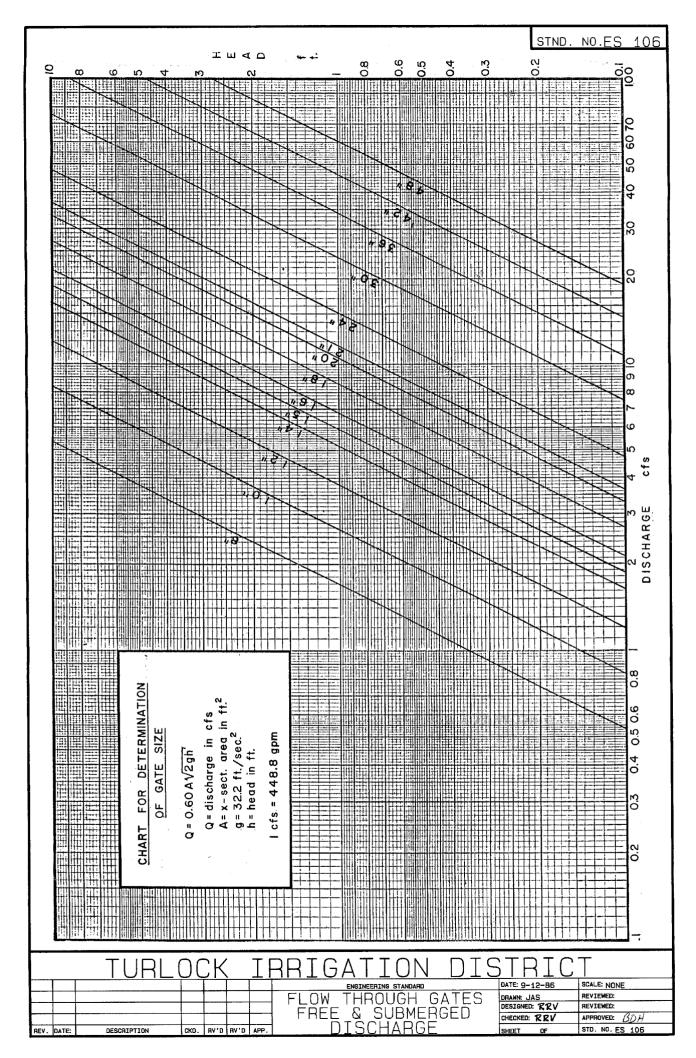
MAXIMUM SIZE OF SURFACE INLETS

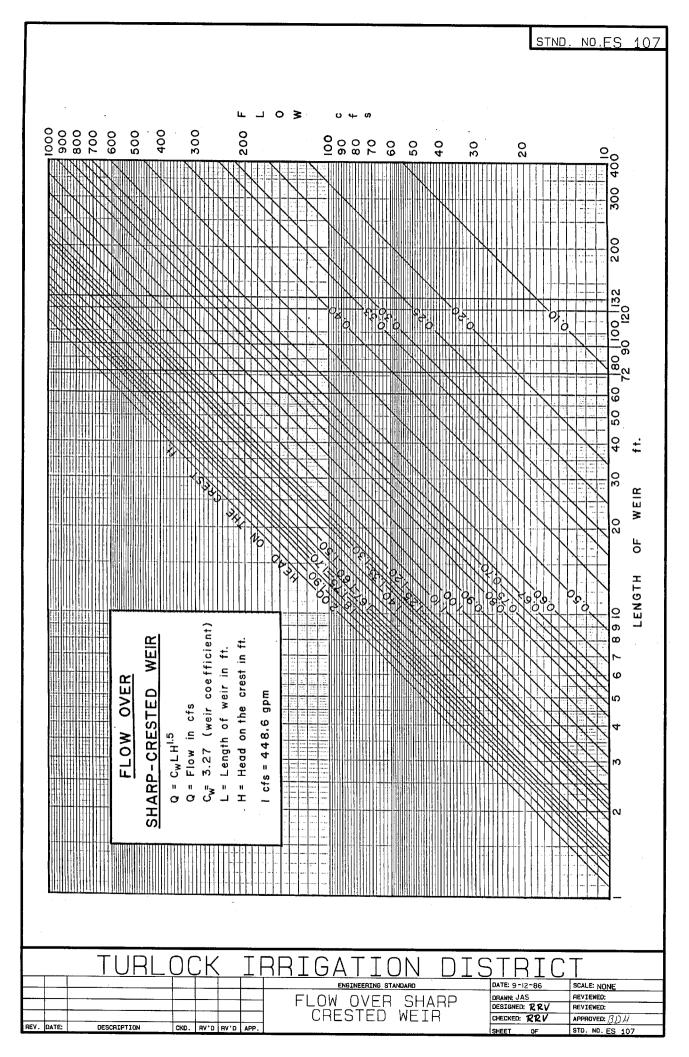
ACRES DRAINED	PIPE SIZE
10	4"
20	4"
40	6"
80	8"
120	10"
160	12"

	D TURLOCK IRR			ADMINISTRA STANDARD							
В	STDS. COM. APPROVAL	GKT	•	9	BK	DB	L.R.				
								DESIGN	STAN	IDARD F	OR
В	CONVERSION TO CAD			CEC	Ab	BZZ	4/00	SURF	'ACE	DRAINS	S
А	ADD WORDING - CLARIFICATION	HBB			(J	BDH	01/27/92				
	INITIAL ISSUE	JAS	RRV			BDH	09/12/86	SHEET	ГC	103B	
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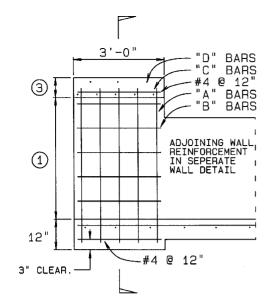


STND NO.ES 108 <u></u>α <u>"</u> "ດ <u>_</u> <u>"</u> <u>0</u> ັ້ ື້ອ <u></u>[ω @ [8] <u></u> <u>"</u> <u>_</u> <u>"</u>0 2.5'-#4@1 DOWEL 4 Ø #40 Ø 3'-#4@ Ð 4'- #40 4 Ø 4 Ø ٩ Ø Ø Ø #4@12" E.W. reinforcement in 8" slab 2.5'-#4 ¥ conc. = 150 pcf 4 3'- #4 4 4 4 c_a = 1/3 2 - #4 # # # # # # # י_ י_ ן_ מ י_ מ 4 ا_ _ 1 ົທ ົທ ٦ • X soil = 120 pcf, Ø=30°, carried through footing. DESIGN ASSUMPTIONS: REINFORCEMENT HORIZONTAL -8 • f'_c = 3000 psi , <u>0</u> ະ ດ <u>[</u>2" @ 12" @ 12" #40 01 ື ຫ <u></u> <u>_</u> 0 <u>_</u> Ξ ື້ອ • $f_y = 40000 \text{ psi}$ Ø Ð O) Ø Ø 0 Ø Ø Ø Ø O **#**4 # 4 #4 4 4 4 **4** # 4 #4 **4** 4 **4** 4 #4 # # n off alternate 4'-0" alternate - 0" f alternate - 0" alternate - 0 off alternate 4' - 0 9 OVERPOURS f 2 out OVERPOUR 1ENGINEERING DESIGN STANDARD Ō <u>0</u> ۲. ۲. ¦ t £4 0 <u></u>44 <u>"</u>0 0 6 ັດ 6. TURLOCK IRRIGATION DISTRICT 4" - cut bars @ 4" - cut bars @ 6" - cut bars @ 6" - cut bars @ 4" - cut bars @ - cut rs @ Ø Q Ø Ø 0 Ð Ø REINFORCEMENT 3 6" - 0 bars # 4 4 4 # 4 44 #4 **4** Ŧ ۱D WEIR # #50 # 2 0 7 જ WEIR 3 ⓓ Ø # 0 い キ 14 # alternate - 0" alternate - 0" 민 19 alternate - 0" ത് WALL every - O WALLS VERTICAL <u>"</u>0 <u>"</u>0 <u>"</u>0 وءً و" <u>†</u>4 <u>"</u>0 <u>_</u> و" £4 Ψū ~ 6 ₽4 - cut rs @ 6" - cut bars @ ≻6" - cut bars @ RETAINING 4" - cut bar © 1 Q 0 0) (1) Ø C 0 Ø Ø bars RETAINING # 4 # 4 4 4 4 # 4 4 4 # ນ ഗ ŧ # # # # 4 ₩ **@** #7@ 0 働 # 10 # 5 ¥ 7 DEPTH STEM 24 24 24 ึง Θ 0 ້ ຍ ัญ 24 6 ____ ัง 2 ω ω HORIZONTAL REINFORCEMENT -VERTICAL REINFORCEMENT G FOOTING THICK-NESS ΰ 16 ັ້ ΰ ัญ ั๊ ัง ົ້ ູ້ທ ΰ ā ัณ N ڡ DOWEL FOOTING WIDTH ō ັດ , 0, <u>0</u>.-`0 ____ <u>ר</u>י 3.5' 4.5' 5.0 ົດ ຸ່ມ ັດ ō ò 1 ณ่ ю <u>ن</u> ~ N ø ω WALL THICK-ື່ຜ ω e ō 6 ω 0 ً ັືໝ <u></u> 0 ົ້ໝ ΰ ั๊ง 2 ² QÌ Ц HEIGHT ABOVE FLOOR Θ 90 19131313131 Θ ัณ m 4 Ξ 4 ē ĩo ~ ัง ī ō œ ັກ R C . ר ENGINEERING STANDARD DATE: 9-12-86 SCALE: NONE REVIEWED: DRAWN: JAS DESIGNED: RRV DESIGN STANDARD FOR RETAINING WALLS & WEIR OVERPOURS REVIEWED: CHECKED: RRV APPROVED: BDU REV. DATE: DESCRIPTION Ско RV'D RV'D APP STD. NO. ES 108

STND. NO.ES 109

STD. NO.ES 109

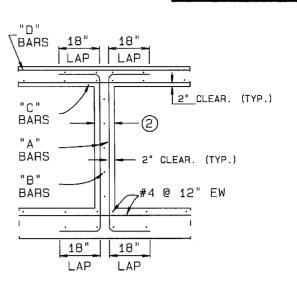
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REV. DATE:

DESCRIPTION



(1) HEIGHT ABOVE FLOOR	(2) WALL THICK- NESS	"A" BARS	"B" BARS	WALK- WAY SPAN LENGTH	Э WALK- WAY THICK.	"C" BARS		"D" BARS				
4 '	6"	#4@10"	#4 @ 18"	4'	8"	3 - #	¥4	3 - #4				
5'	6"	#4@9"	#4 @ 18"	5'	8"	3 - #	¥4	3 - #4				
6,	8"	#4@6"	#4 @ 12"	6,	8"	4 - 7	¥4	3 - #4				
7'	8"	#4@6"	#4 @ 12"	7'	8"	5 - 7	# 4	3 - #4				
8'	8"	#5@6"	#4 @ 12"	8'	8"`	6 - 7	# 4	3 - #4				
д,	10"	#5@6"	#4 @ 10"	Э,	8"	5 - 7	# 5	3 - #4				
10'	10 "	#6@10"	#4 @ 10"	10'	8"	6 - ;	# 5	3 - #4				
11'	10 "	#6@10"	#4 @ 10"	11'	8"	5 - 1	# 6	3 - #4				
12'	12"	#6@6"	#4@9"	12'	8"	6 - 1	#6	3 - #4				
13'	12"	#6@6"	#4@9"	13'	10 "	6 - ;	#6	3 - #5				
14'	12"	#6@6"	#4@9"	14'	10"	5 - ;	#7	3 - #5				
DESIG	GN AS	SUMPTIONS		15'	10"	6 - ;	#7	3 - #5				
F,	= 3000	PSI, X CONC.	= 150 PCF	16'	10 "	5 - :	#8	3 - #5				
	= 40000	PSI W. REINFORCE	AENT TN Q ¹¹	17'	10"	6 - :	#8	4 - #5				
		RRIED THROUGH		18'	10"	7 - :	#8	4 - #5				
DES	IGN LIVE	E LOAD = 600 I	_BS	19'	10"	7 -	#8	4 - #5				
SPA	•	PLE @ 200 LBS	·	20,	10"	8 -	#8	5 - #5				
	SPAN LENGTH: & TO & OF SUPPORTS											
	TUBLOCK TRBIGATION DISTRICT											
				ENGINEERIN	S STANDARD		DATE: 9-12-8					
					STAND/	ARD 🛛	DRAWN: JHS DESIGNED: R					
				CANAL	_ WALK	KWAY	CHECKED: R	RV APPROVED: BDH				
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UNDERGROUND FACILITIES CROSSING DISTRICT IRRIGATION FACILITIES

Revocable License Agreement

Facility owners wishing to cross District facilities shall request and obtain a revocable license agreement from the District prior to constructing improvements within the District's right-of-way.

Minimum Clearance and Posting

All facilities crossing District irrigation facilities shall provide 24" minimum clearance below District facilities. All facilities crossing District irrigation facilities shall have a sign (supplied and installed by the District) posted on the canal bank. (See page two for details)

Timing of Construction

Any construction within the Turlock Irrigation District right-of-way that may affect the flow of water in the District's canals and/or pipelines can not be undertaken during the irrigation season which is generally from March 1 to November 1 unless approved by the Turlock Irrigation District.

Required Inspections

The District must be notified a minimum of 2 working days in advance of any construction affecting District facilities in order to schedule the required inspections.

Lined Canals

Crossings under lined canals shall be drilled or bored and jacked in-place.

Unlined Canals or Drains

Unlined canals or drains may be open-cut during the off-season. A six inch minimum thickness unreinforced concrete protection slab shall be placed a minimum of 24 inches below the invert of the canal. The width of the concrete slab shall be the outside dimension of the utility cable or pipeline plus 36 inches and extend the bottom width of the canal. The backfill shall be compacted to 95% maximum density. Compaction tests in accordance with ASTM D-1557 shall be performed and the results provided to the District. The operating roadway and banks shall be replaced to the original condition.

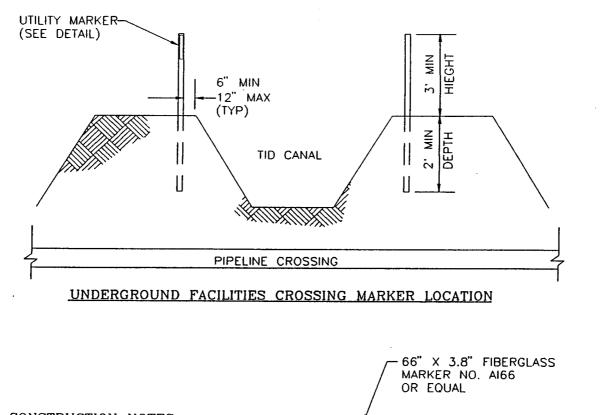
Crossings Utilizing a Casing

When a casing is used, it must extend from outside of embankment to outside of embankment. The casing shall have a wall thickness of 1/4" for galvanized or 3/8" non-galvanized steel pipe. The casing pipe minimum diameter shall be the utility cable or pipe outside dimension at the bell plus six (6) inches. The utility cable or pipe shall be firmly attached to redwood skids with steel straps for installation. After all casing pipe and utility cable or pipe is installed, ends of casing shall be closed with redwood bulkheads closely fitted around the utility cable or pipe.

Direct Installation of Crossing Facility

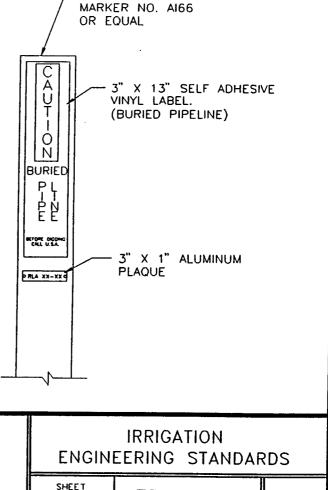
Plastic pipe with a standard dimensional ratio (SDR) of 41 or less, or Plastic Irrigation Pipe (PIP) of 100 psi or higher rating may be installed from outside bank to outside bank without casings. The pipe sections shall be joined with elastomeric ring slip couplings. Other materials may be used with the approval of the District.

7///		IRRIGATION ENGINEERING STANDARD								
						<u> </u>			DERGROUND	
В	STANDARDS COMMITTEE APPROVAL	3B	6	BKA	ν	ĮΔR,	1-8-97	FACIL	ITIES CROSSIN	IG
									ICT IRRIGATIC	N
В	Revised wording to include const. specs	BB			BOH	B22	1-9-97]]	FACILITIES	
A	Revised wording for clarification	HBB				BDH	2-21-92			
	Initial Issue	JAS	RRV			BDH	9-12-86	SHEET	ES 110 B	
REV	DESCRIPTION	INIT	СНК	RVD	APP	APP	DATE	1 of 2	DWG NO.	PAGE



CONSTRUCTION NOTES

- 1. PIPELINE CROSSING MARKER SHALL BE PLACED NO MORE THAN 12 INCHES FROM THE EDGE OF LINING OR CANAL.
- 2. THE MARKER POST SHALL BE FIBERGLASS MARKER NO. AI66 FROM SAFETY LITE OR EQUAL.
- 3. THE MARKER POST VINYL LABEL SHALL BE "BURIED PIPELINE" TYPE FROM SAFETY LITE OR EQUAL.
- 4. PIPELINE CROSSING MARKER SHALL HAVE A 3" X 1" PLAQUE CONTAINING THE REVOCABLE LICENCE NUMBER FOR FUTURE REFERENCE.



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DWG. NO.

UNDERGROUND FACILITIES CROSSING DISTRICT IRRIGATION FACILITIES

REQUIRED EASEMENTS AND RIGHTS-OF-WAY

- 1. 25 foot wide easement for irrigation pipelines, pumplines, spill lines, drain lines and subsurface drainage lines, centered on the pipeline.
- 2. 40 foot wide easement for irrigation and drainage ditches, lined or dirt bank, centered on the ditch.
- 3. 15 foot wide easement from road and canal rights-of-way for irrigation or drainage pipelines whose center line is less than 12 ½ feet from the right-of-way.
- 4. 10 foot wide easement from rights-of-way for pipelines within developments that are parallel to and adjoining the street right-of-way line behind the sidewalk. If the same area is to be occupied with other utilities (joint utility trench) 15 feet is required.
- 5. 50 foot square easement for drainage and irrigation pumps, centered on the pump. A 20 foot access easement from a public road to the pump is required.
- 6. 12 foot wide access easement to control structures and access manholes. Access easements to control structures shall be free of encroaching structures and consist of an all-weather surface.
- 7. Easement for canals are determined by the width needed to accommodate the standard roadway and embankment width (CS 127) on both sides of canal, plus the canal width necessary to convey the designed flow rate.

7//		IRRIGATION CONSTRUCTION STANDARDS									
В	B STANDARDS COMMITTEE APPROVAL BB S.R. BBCDB 5-11-99										
				00				REQUIRED EASEMENTS			
В	Revised Wording				9	BZZ	5/11/ga	AND RIGHTS-OF-WAY			
Α	Wording Changes	HBB			~	BDH	3/23/92				
	Initial Issue	JAS	RRV			BDH	9/12/86	SHEET	ES 111 B		
REV	DESCRIPTION	INITI	СНК	RVD	APP	APP	DATE	1 OF 1	DWG NO.	PAG E	

STND. NO. ES 112

<u>IRRIGATION</u> <u>PUMP AND WELL</u> <u>STANDARDS</u>

THIS STANDARD CONSISTS OF THE FOLLOWING PAGES:

COVER	1 PAGE
TEXT	10 PAGES
CS 122	1 PAGE
CS 141	2 PAGES

		TURL	00	<u>СК</u>	·	IF	RRIGATION DIS	STRIC	Т
				<u> </u>			ENGINEERING STANDARD	DATE: 3-21-89	SCALE:
							IRRIGATION PUMP AND WELL	DRAWN: AWV DESIGNED: WBF	REVIEWED: REVIEWED:
REV.	DATE:	DESCRIPTION	СКD.	∄V⁺D	RV'D	APP.	STANDARDS	CHECKED: SHEET 1 OF 14	APPROVED: (O) Gues STD. NO. ES 112

Date: March 30, 1989

TURLOCK IRRIGATION DISTRICT IRRIGATION PUMP AND WELL STANDARDS

Preamble

The intent of these standards is to insure that wells used to provide the Turlock Irrigation District (hereinafter District) with water or to control groundwater levels are constructed using high quality methods and materials that will assure a sound well with high quality water that will protect groundwater resources, and will serve the long term best interests of the District, and meet county and state standards.

- I. Well Driller
 - A. The well driller shall have a valid California State Contractors License A or C-57 issued in accordance with the provisions of the Contractors License Law of the State of California (Chapter 9, Division 3, business and professions code).
 - B. Minimum of three years experience is recommended.
 - C. Minimum of three references.
 - D. Well driller should be familiar with local soil conditions.
 - E. Well driller must obtain a drilling permit from County Health Department and comply with all applicable county and state standards.

II. Location

- A. The proposed irrigation well shall not be closer than 3 mile from any other irrigation deep well, District drainage well, or municipal supply well.
- B. Shall meet County set back requirements.
- C. Should be adjacent to District canal bank such that water may be delivered to either the canal or the head of a pipeline and meet space requirements in Section V J. An alternative location may be considered by District.

III. Analysis of Existing Conditions

Recent groundwater studies by the District give a generalized level of information regarding anticipated

salinity levels for proposed wells. Analysis of existing wells within a mile of the proposed well will only give an indication of anticipated groundwater salinity problems. Use of this information in constructing a new well will not assure that saline groundwater problems, Ecw greater than 1.5, will not develop and that remedial work to correct salinity problems will not have to be performed. The final well must have a salinity level less than Ecw of 1.5.

IV. Test Hole Requirements

Aquifers with bad water quality can be located by a test hole and provisions can then be planned to seal those aquifers during full scale construction of the well. The purpose of a "test hole" is to provide design and construction information necessary to reasonably assure that the water quality in the completed well will have a salinity level Ecw less than 1.5.

- A. A test hole is required prior to drilling and constructing a new well. Pumping strata with water having an Ecw greater than 1.5 shall be identified.
- B. Driller shall provide the District with the test hole drilling log and Ec log. A sieve analysis should be made from the aquifers to be used to supply the water.
- C. Prior to constructing the main well, the well driller will submit to the District for review and comment his proposal on well drilling, casing installation, sealing of strata with water quality exceeding an Ecw of 1.5, and gravel pack grading where a rotary dug well is to be installed.
- V. Standards for well construction

The well to be constructed may be either a closed bottom gravel packed rotary dug well or an open bottom cable tool dug well. The well driller shall conform to the below District standard applicable to the drilling method used.

- A. Well Type: Gravel Packed Rotary
 - 1. The well is to be straight such that the pump can be installed with adequate clearance and true such that the pump shaft will align with a level motor base.
 - 2. The well shall be gravel packed with a closed bottom and the preferred drilling method shall be reverse rotary. Alternative drilling methods require prior approval by T.I.D.
 - 3. Well casing shall be standard or line grade S" thick meeting ASTM A139 or A135. To minimize corrosion, the

Irrigation Pump and Well Standards Page 3 March 30, 1989

steel should have .2% copper.

- 4. A 16 to 18 inch diameter casing recommended.
- 5. Well must be solid cased to first water bearing strata below 50' with a neat cement-sand grout seal placed from the bottom up with a tremie pipe for the first 50 feet. Drilling mud or Bentonite may not be used to seal the conductor casing.
- 6. Well shall be landed on clay with a minimum of 10 feet of blank casing capped on the bottom.
- 7. A minimum of 18" of casing to remain above natural ground prior to constructing the pad foundation when pad construction to be by T.I.D. Weld a temporary protective cap cover over the open casing until the pump is installed.
- 8. Perforations should be no larger than 3/16", machine made louver form, facing down, perpendicular to the well axis, and made of the same material as the casing. Alternative types of screens or perforations require prior approval by the District.
- 9. The gravel pack shall be washed, natural, well rounded pea gravel containing no silt or clay, no larger than 5/16", and graded such that 70% retained of the gravel pack is between 4 and 9 times the 70% retained size of the finest portion of the aquifer. The coefficient of uniformity for the gravel pack should be equal to or less than 2.50. Crushed rock may not be used for the gravel pack.
- 10. Overbore should be minimum of 8" larger than well casing to minimize problems in placement of gravel pack. The gravel shall be placed in a slow steady manner using equipment and methods to eliminate voids and bridging within the gravel pack.
- 11. Conductor casing should be aluminum or galvanized corrugated metal.
- 12. Conductor casing shall be a minimum of 50' deep and landed on clay if possible.
- 13. Conductor casing should be a minimum of 12" larger than casing and should provide a minimum of 6" between the

Irrigation Pump and Well Standards Page 4 March 30, 1989

casing and conductor casing to minimize bridging in the gravel pack.

- 14. A minimum of 12" of conductor casing should remain above natural ground and the grout seal shall be visible prior to placing foundation slab.
- 15. A 2" galvanized sounding tube, welded at a 45" angle to the casing with no burs, and a 10" gravel fill pipe, attached at a 45" angle to the conductor casing shall be provided. The exposed ends are to have a removable cap.
- 16. The space between the casing and pump head shall be sealed with a compressible gasket or non-shrink grout seal.
- 17. Well driller to notify the District a minimum of 48 hours before starting to drill well.
- B. Well Type: Cable Tool
 - 1. The well is to be straight such that the pump can be installed with adequate clearance and true such that the pump shaft will align with a level motor base.
 - 2. The well shall be blank cased with an open bottom and the drilling method shall be by cable tool.
 - 3. Well casing shall be standard or line grade ¼" thick meeting ASTM A139 or A135. To minimize corrosion, the steel should have .2% copper.
 - 4. A 16 to 18 inch diameter casing recommended.
 - 5. Well must be solid cased to first water bearing strata below 50' with a neat cement-sand grout seal, placed from the bottom up with a tremie pipe, for the first 50 feet. Drilling mud or Bentonite may not be used to seal the casing.
 - 6. Well casing should be landed in clay a minimum of 10 feet.
 - 7. A minimum of 18" of casing to remain above natural ground prior to constructing the pad foundation when pad construction to be by T.I.D. Weld a temporary protective cap cover over the open casing until the

Irrigation Pump and Well Standards Page 5 March 30, 1989

pump is installed.

- 8. Overbore should be a minimum of 6" larger than well casing to minimize problems in placement of the grout seal.
- 9. Temporary conductor casing shall be a minimum of 50' deep and landed on clay if possible to insure the grout seal reaches the 50 foot depth. The conductor casing shall be withdrawn as the grout seal is placed from the bottom up.
- 10. Temporary conductor casing should be a minimum of 6" larger than well casing and should provide a minimum of 3" between the well casing and conductor casing to minimize bridging when the grout seal is placed.
- 11. The grout seal shall be visible prior to placing foundation slab.
- 12. A 2" galvanized sounding tube, welded at a 45" angle to the casing with no burs, shall be provided. The exposed end is to have a removable cap.
- 13. The space between the casing and pump head shall be sealed with a compressible gasket or non-shrink grout seal.
- 14. Well driller to notify the District a minimum of 48 hours before starting to drill well.
- C. In addition to the above well construction standards, those applicable portions of the current California Department of Water Resources Water Well Standards", Bulletin 74, shall be adhered to. More restrictive county standards and regulations regarding well construction shall be adhered to.
- D. Well Development
 - 1. The well shall be developed by the drilling company after completion of drilling using equipment with a capacity of 1.5 times the planned well output. After development is complete, a sand test shall be made. The test shall consist of pumping the well for 30 minutes at 1.5 times the planned design capacity. Sand production greater than 10 ppm may require additional development or modifications to pump equipment to reduce wear from sand.

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- 2. To assess the capacity of the well, a four point drawdown test should be made with pumping points evenly spread between 0 and 1.5 times the well design capacity. Pumping should continue at each rate for a sufficient length of time to bring about a relatively steady water level in the well. The well driller should determine the length of time for well recovery after final drawdown.
- 3. The District Hydrographer will verify the output of the developed well and the output curve supplied by the drilling company. The hydrographers shall be notified 24 hours prior to the start of well development.
- 4. A water quality test (irrigation panel) will be taken by the District staff at the completion of development.
- E. Pump
 - 1. The District shall review the pump purchase specifications and make recommendations to the pump installer.
 - 2. The installation of the pump shall be such that the shaft is straight and that the column is assembled correctly.
 - 3. The pumping plant must meet a minimum of 50% overall efficiency, with a motor overload not exceeding manufactures recommendations.
- F. Electrical Panel
 - 1. District to stake location for service pole.
 - Panel to be located on service pole a minimum of 6' from well.
 - 3. Bottom of panel to be a minimum of 18" above natural ground.
 - 4. Wiring from panel to pump to meet county standards.
 - 5. A part wind panel is required for motors 40 H.P. and up.
 - 6. Panel must have 3 leg protection.
 - 7. Locking devices must be on panel and main switch. 8.

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All county codes to be met.

8. All county codes to be met.

G. Foundation

A slab type foundation shall be used and constructed to District construction standards, drawing No. CS141A or CS141B.

- 1. The pump foundation slab shall be a minimum of 10 foot square with the top 4" above finished grade.
- 2. The finish grade of the pump head shall be a minimum of 1 inch above the top of the slab.
- 3. The joint between the casing grout seal and the foundation slab shall be clean and free of grease or loose material prior to pouring the foundation slab to pouring the foundation slab to insure a good seal.

H. Discharge pipe

- 1. Discharge shall have a minimum of 10 gage thickness.
- 2. Discharge pipe from pump into canal shall be constructed to the District standard CS 122.
- 3. Spillover discharges into canal should be constructed at the time of pump installation and built to District standard CS 122.
- 4. Pumps discharging into Improvement District pipelines may require raising improvement district structures at the time of pump installation.
- 5. The discharge line shall have a minimum of 10 diameters of straight, level pipe between the pump head and the control valve. A 3/4" brass testing plug shall be installed in a 304 stainless steel female pipe bung in the 2 o'clock position, a distance of 8 pipe diameters downstream of the pump head.
- 6. Discharge control should be gate valves or geared butterfly valves, using mechanical connectors, one of

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which shall be a flanged couple adapter. The valve should be supported.

- 7. The pump discharge should discharge into District canal.
- I. Safety

Construction will be done in compliance with county and state safety codes.

- J. Easement
 - 1. A 20' access easement is required for access to well site.
 - 2. A 50' X 50' Grant of Easement to the District is required for the well site.
 - 3. A chain link fence enclosure may be placed around the pump and drop pole and built to District Standards.
 - 4. The well shall be located off the bank of the District canal.
 - 5. An easement appropriate for appurtenant pump and well facilities shall be provided.
- K. Building. Any pump house building should be built to the District construction standards.
- VI. Abandonment of Wells

During the life of a well, its condition may deteriorate beyond what is economical or feasible to rehabilitate, its use may no longer be needed, or groundwater conditions change rendering the well unusable and the District makes the determination that the well is to be abandoned.

All "abandoned" wells and exploration or test holes shall be destroyed. The objective of destruction is to restore as nearly as possible those surface conditions that existed before the well was constructed and taking into account changes that have occurred since the well was constructed. Irrigation Pump and Well Standards Page 9 March 30, 1989

A. Preliminary Work

Prior to destroying the well, it will be investigated to determine its condition, the location of any obstructions that may interfere with filling or sealing, and any special circumstances that may be required for the well's destruction.

- 1. Obstructions will be removed if possible.
- 2. A down hole television is recommended to determine the well condition.
- B. Filling and Sealing Conditions

Following are sealing and filling requirements to be practiced when the conditions below are encountered:

- 1. For a well wholly situated in unconsolidated material in an unconfined groundwater zone, the upper 20 feet shall be sealed with impervious material and the remainder of the well shall be filled with clay, sand, or other suitable inorganic material.
- 2. For a well penetrating several aquifers or formations, the upper 20 feet of the well shall be sealed with impervious material.

In areas where the interchange of water between aquifers will result in a significant deterioration of the quality of water in one or more aquifers, or will result in a loss of artesian pressure, the well shall be filled and sealed so as to prevent such interchange. To prevent the vertical movement of water from the producing formation, impervious material must be placed opposite confining formations above and below the producing formations for a distance of 10 feet or more. The formation producing the deleterious water shall be sealed by placing impervious material opposite the formation, and opposite the confining formations for a sufficient vertical distance, but no less than 10 feet in both directions, or in the case of "bottom" waters, in the upward direction.

In locations where interchange is in no way detrimental, suitable inorganic material may be placed

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opposite the formations penetrated. When the boundaries of the various formations are unknown, the well casing shall be filled entirely with sealing material.

C. Placement of Material

The following requirements shall be observed in placing fill or sealing material in wells to be destroyed:

- 1. The well shall be filled with the appropriate material (as described in item D of this section) from the bottom of the well up.
- Where neat cement grout, sand-cement grout, or concrete is used, it shall be poured in one continuous operation.
- 3. Sealing material shall be placed in the interval or intervals to be sealed by methods that prevent free fall, dilution, and/or separation of aggregates from cementing materials.
- 4. Where the head (pressure) producing flow is great, special care and methods must be used to restrict the flow while placing the sealing material. In such cases, the casing must be perforated opposite the area to be sealed and the sealing material forced out under pressure into the surrounding formation.
- 5. In destroying gravel-packed wells, the casing shall be perforated or otherwise punctured opposite the area to be sealed. The sealing material shall then be placed within the casing, completely filling the portion adjacent to the area to be sealed and then forced out under pressure into the gravel envelope.
- 6. When pressure is applied to force sealing material into the annular space, the pressure shall be maintained for a length of time sufficient for the cementing mixture to set.
- 7. To assure that the well is filled and there has been no jamming or "bridging" of the material, verification shall be made that the volume of material placed in the well installation at least equals the volume of the empty hole.

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D. Materials

Requirements for sealing and fill materials are as follows:

1. Impervious Sealing Materials. No material is completely impervious. However, sealing materials shall have such a low permeability that the volume of water passing through them is of small consequence.

Preferred impervious materials include neat cement, sand-cement grout, concrete. Native material or sand silt clay mixtures with a coefficient of permeability less than 10 feet per year may be used. Drilling muds shall NOT be used.

- 2. Filler Material. Many materials are suitable for use as a filler in destroying wells. These include clay, silt, sand, gravel, crushed stone, native soils, and mixtures of the aforementioned types. Material containing organic matter shall not be used.
- E. Sealing Wells in Urban Areas

In incorporated areas or unincorporated areas developed for multiple habitation, to make further use of the well site, the following additional requirements must be met:

- 1. A hole shall be excavated around the well casing to a depth of 5 feet below the ground surface and the well casing removed to the bottom of the excavation.
- 2. The sealing material used for the upper portion of the well shall be allowed to spill over into the excavation to form a cap.
- 3. After the well has been properly filled, including sufficient time for sealing material in the excavation to set, the excavation shall be filled with native soil.
- F. Temporary Cover

During periods when no work is being done on the well, such as overnight or while waiting for sealing material to set, the well and surrounding excavation, if any, shall be covered. The cover shall be sufficiently strong and well enough anchored to prevent anchored to prevent the introduction of foreign material into the well and to Irrigation Pump and Well Standards Page 12 March 30, 1989

protect the public from a potentially hazardous situation.

G. In addition to the above well abandonment standards, those applicable portions of the current California Department of Water Resources "Water Well Standards", Bulletin 74, shall be adhered to. More restrictive county standards and regulations regarding well abandonment shall be adhered to.

Rev. 8/18/93

ID 1019

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CONDITIONS FOR DOWNSIZING IRRIGATION FACILITIES

Irrigation facilities may be designed and installed for a rate of flow of less than the standard irrigation flow of 15 CFS if the following conditions exist:

- 1. The irrigated parcels historically use less than the standard flow or changes in land use, cropping or other factors make the use of a smaller flow reasonable and practical for the remaining lands being irrigated.
- 2. The reduced flow will not unreasonably affect the:
 - a. Operation of the District's Irrigation System.
 - b. Operation of the affected Water Distribution Service Area.
 - c. Operation of the affected Improvement District or private facilities.
 - d. The customer's irrigation practices.

The standard flow rate for downsized facilities is 5 CFS or historical usage of the affected parcels.

	TURLOCK IRRIGA	TIC)N [IRRIGATION SYSTEM ADMINISTRATION ENGINEERING STANDARDS								
								CONDITIONS FOR DOWNSIZING IRRIGATION FACILITIES				
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WATER QUALITY STANDARD FOR DISCHARGE TO CANAL SYSTEM

Constituent	Unit	Maximum Concentration			
Sodium (Na)	meq/l	3			
Chloride (Cl)	meq/l	4			
Conductivity (EC)	μmhos	1400			
Nitrates (NO3)	ppm	200			
Boron (B)	ppm	0.7			
Adj. SAR		<3.0 @ EC=1000 <6.5 @ EC=1400			
Sand Production (sediment)**	ppm	10			
рH		6.5 - 8.4			

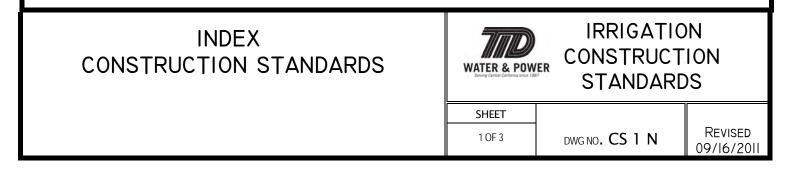
NOTE:

Municipal and Industrial discharges may have other constituents that could impact the District's ability to allow such discharges into the facility.

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CONSTRUCTION STANDARDS INDEX

Dwg #	# OF Sheets	Title	Date	REVISION
CS 050	2	CONSTRUCTION & DEVELOPMENT STANDARD SPECIFICATIONS	07/00/10	С
CS 060	-	DRIVEWAY PROTECTION FOR EXISTING CONCRETE PIPE	10/00/99	B
CS 099	ĺ	PLASTIC PIPE DETAIL	07/00/10	D
CS 100		Concrete Pipe Detail	05/11/99	С
CS 101	ĺ	Typical Control Box	07/00/10	D
CS 102		CONCRETE COLLAR FOR IRRIGATION PIPELINES	07/00/10	B
CS 103	3	SPRINKLER IRRIGATION CONNECTION TO LATERAL	01/00/04	В
CS 104	2	CANAL SIDEGATE FOR IRRIGATION PIPELINE	06/00/98	С
CS 106	I	EXPANSION JOINT DETAIL	02/00/88	
CS 107	I	Access and Air Vent	07/00/10	С
CS 108	I	PUMP LINE CROSSING OVER CANAL	10/00/99	В
CS 109	3	SMALL CANAL SIDEGATE FOR IRRIGATION PIPELINE	01/06/03	А
CS 110	I	Weir-Board Slide Gate	12/00/97	А
CS II3	I	METAL GATE FOR CANAL ROADS	12/00/03	D
CS 114	I	DOUBLE BOX STRUCTURE	05/01/87	Ι
CS II5	I	GRATE FOR CANAL SIDEGATE WITH VERTICAL WALLS	10/00/98	А
CS II6	I	24" CHECK GATE AND U-BOX STRUCTURE	07/00/10	В
CS II7	I	REINFORCED CONCRETE PIPE PLUG	07/00/10	В
CS II8	I	SEALING CHECK GATE IN DITCH	05/01/87	Ι
CS 119	I	CONCRETE COLLAR FOR SMALL DIAMETER PIPE	12/08/97	А
CS 120	I	PIPELINE DISCHARGE FROM LINED DITCHES	06/13/83	
CS 121	I	CONNECTION BETWEEN PIPE AND CONCRETE LINED DITCH	07/00/10	В
CS 122	I	Pump Discharges into Canal	05/11/99	С
CS 123	2	GRATE FOR CANAL SIDEGATE WITH SLOPED LINING	12/00/03	С
CS 124	I	COVER FOR SMALL DIAMETER CONCRETE VENTS	10/10/85	Ι
CS 125	2	GATED DRAIN LINE INTO TID DRAIN DITCH	07/00/10	А



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Dwg #	# OF SHEETS	Title	DATE	REVISION
CS 126	I	CANAL STRUCTURE GUARDRAIL	04/00/87	2
CS 127	I	LATERAL AND ROADWAY CONSTRUCTION	07/00/10	В
CS 128	I	GALVANIZED LADDER FOR CONTROL STRUCTURE	08/19/97	А
CS 129	I	CONTROL STRUCTURE WITH WEIR OVERPOUR	07/00/10	В
CS 130	2	EXPANDED METAL COVER FOR CONTROL BOX	07/00/10	С
CS 133	I	Concrete Bridge	12/00/03	А
CS 134	I	Reverse Sidegate	07/00/10	С
CS 135	5	RECORDER STATION STRUCTURE	07/00/10	D
CS 136	I	GRATE MODIFICATION FOR BOOSTER PUMP INTAKE	01/13/86	
CS 137	I	DEBRIS GRATE FOR BOOSTER PUMP INTAKE	01/12/86	
CS 138	I	Concrete Weir Board Box	02/05/87	
CS 139	I	FIXED METAL LADDER FOR ROOF ACCESS	09/03/87	
CS 141	I	PAD FOUNDATION FOR GRAVEL-PACKED WELL	02/02/89	
CS 141B	I	PAD FOUNDATION FOR CABLE TOOL WELL	02/02/89	
CS 142	3	PUMP HOUSE FRAME DETAILS AND PUMPING PLANT SITE PLAN	06/02/89	
CS 143	I	IMPROVEMENT DISTRICT DITCH DESIGN	02/21/92	
CS 144	I	AIR VENT INSTALLATION	07/00/10	В
CS 145	I	DROP INLET	08/19/97	А
CS 146	I	DROP INLET INTO BOX	08/19/97	А
CS 147	3	CUSTOMER INSTALLED CANAL SIDEGATE FOR IRRIGATION PIPELINE	12/00/03	А
CS 148	4	SAW-CUT LINING CONSTRUCTION	06/00/98	
CS 149	I	AIR VENT FOR PVC PIPE	12/00/03	
CS 156	I	Pressure Box and Manhole Access	07/00/10	D
CS 157	I	SAFETY GRATE FOR SIPHON	07/08/97	
CS 158	I	Small Diameter Pressure Pipe	08/00/99	
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	2 OF 3	DWG NO. CS 1 N	REVISED 09/16/2011		

	CONSTRUCTION STANDARDS INDEX										
Dwg #	# OF SHEETS	Title	DATE	REVISION							
CS 162	4	Canal Walkway and Handrail	04/07/00								
CS 163	I	THRUST BLOCKS	12/00/03								
CS 164	2	CANAL FENCE AND GATES	07/00/04								
CS 165	I	PIPE CROSSING BARB WIRE FENCE	07/00/10								
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CS 167	Ι	PARALLEL PIPELINE IN CANAL ROW APPLICATIONS	03/00/10								

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	SHEET					
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- At least 30 calendar days prior to the commencement of any work to remove existing irrigation facilities or to construct new irrigation facilities, the developer shall sign the irrigation improvements agreement with Turlock Irrigation District (TID) and provide the two required improvement securities and the required public liability and property damage insurance coverage.
- 2. Developments adjoining TID canals shall construct a solid masonry or concrete wall, a minimum of six feet in height, next to the TID right—of—way as per TID Construction Standard CS 166.
- 3. Lots adjoining irrigated ground must be graded so that the backs of lot and house pad elevations are at least 6 inches higher than the adjoining irrigated ground.
- 4. Contractor shall verify pipe sizes and inverts prior to construction of irrigation facilities.
- 5. Contractor must furnish a detailed construction and inspection schedule for TID written approval prior to excavation or construction within district rights-of-way or easements. Irrigation service must be maintained during the irrigation season, which is generally between March 1 through October 31, but can vary.
- 6. TID reserves the right to construct all structures within developments. If TID constructs the structures, the developer shall provide a deposit to the TID for the estimated construction costs. The pipeline contractor shall leave an open space of 6 feet minimum and 8 feet maximum at each structure location. The opening shall be formed and finished or sawn; construction of the opening by impact methods is not allowed.
- 7. All construction of TID irrigation facilities shall be done in accordance with the California Building Code, other recognized national standards, and TID standards and specifications. TID standards and specifications shall govern in the event of a discrepancy.

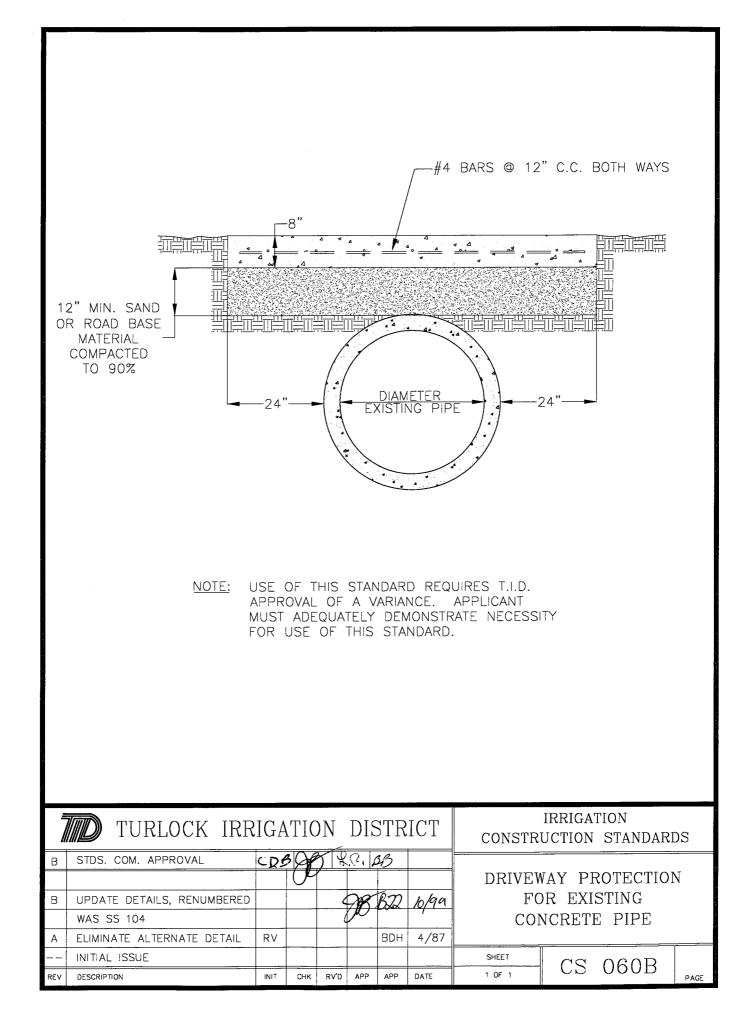
8. All irrigation structure boxes shall be formed inside and out and concrete vibrated sufficiently to provide for smooth surface walls without voids and honeycombs. Waterstop (Waterstop RX or approved equal) shall be used at all cold joints and shall be installed in accordance with the instructions recommended by the manufacturer.

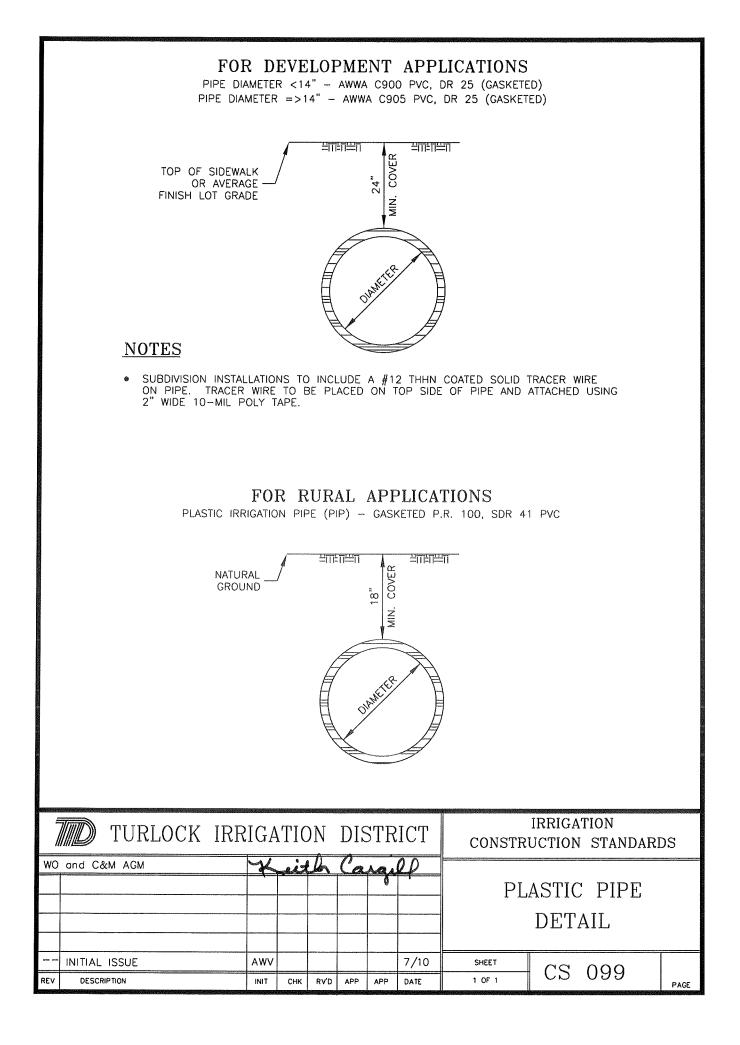
- 9. Concrete shall be 3,000 psi or stronger at 28 days. Upon request by TID, concrete compressive tests on irrigation facility construction will be done by contractor at contractor's expense. It is the contractor's responsibility to deliver concrete samples to the concrete lab and send the results to TID.
- 10. All earthwork for irrigation facilities shall achieve a minimum relative compaction of 90% in rural areas and 95% in developed areas, according to ASTM D-1557. City, County and State compaction standards may, in some cases, supercede the above standards. Upon request by TID, test results verifying this shall be furnished to the TID by any contractor engaged in this work for the TID.

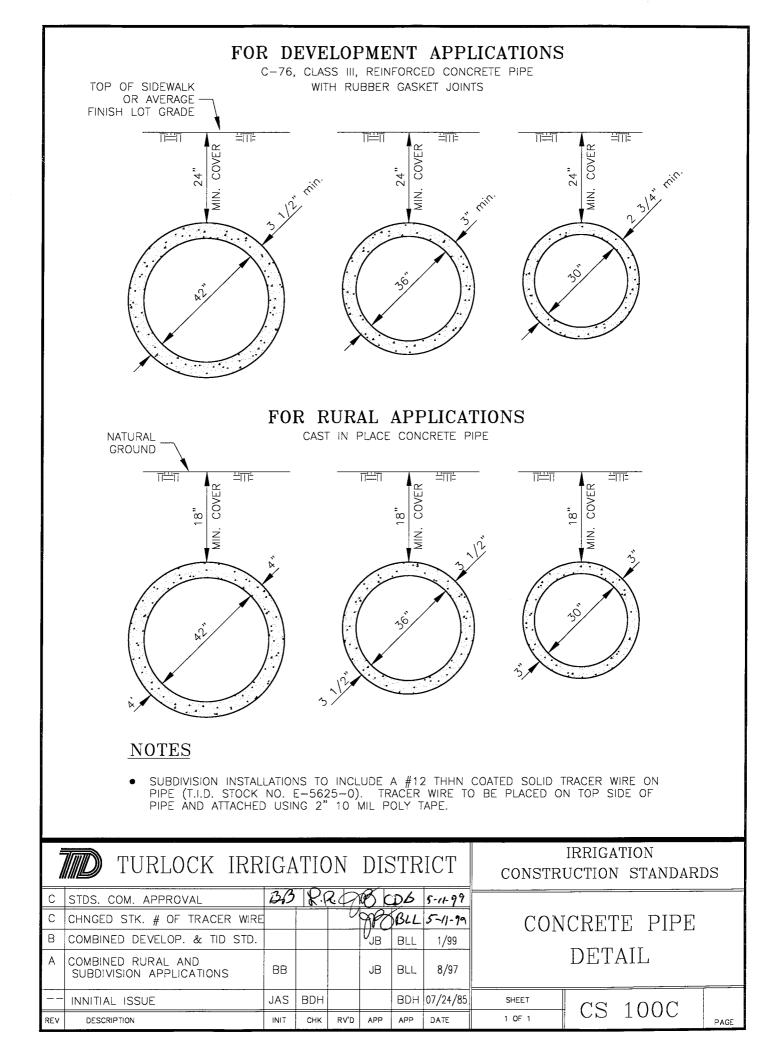
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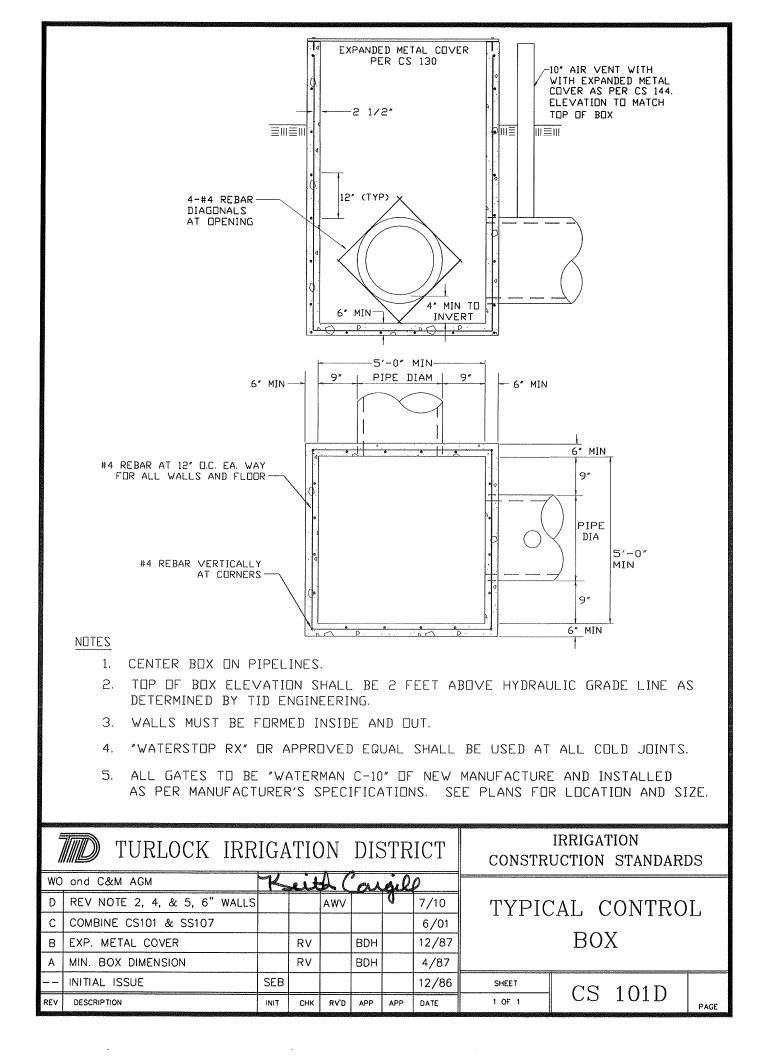
- 11. Prior to working on TID facilities, contractors are responsible for contacting Underground Service Alert for utility locations at 800-227-2600.
- 12. Inspections must be requested two working days in advance. The TID's inspector shall inspect all work phases on irrigation facilities for conformance to approved engineering plans and TID specifications. Reinforcing shall not be encased in concrete without prior TID inspection and approval. Likewise, concrete structures and pipelines shall not be covered with earth prior to TID inspection and approval.
- 13. Control structures and access manholes shall be constructed adjacent to public rights-of-way. If no public access is available, an access easement from a public right-of-way, a minimum of 12 feet in width, shall be provided to all control structures and access manholes. Access easements to control structures shall be free of encroachments, accommodate vehicle access, and must have an all weather surface.
- 14. Fences within irrigation easements must be constructed to allow access to irrigation facilities, as directed by TID.
- 15. All irrigation facilities shall pass the pressure test described in Caltrans Standard Specification 65-1.08. Upon request by the TID, the test will be performed by contractor at contractor's expense. The tested head shall be the maximum operational pressure of the line which will be supplied by the TID. The testing shall be observed and certified by a licensed Civil Engineer.
- 16. Irrigation facilities which are determined, by TID, as no longer needed shall be removed prior to development.
- 17. All pipeline installations shall include a #12 THHN coated solid tracer wire (TID Stock No. E-5625-0) on pipe. Tracer wire to be placed on top side of pipe and attached using 2" 10 mil poly tape or acceptable alternative.
- 18. Used materials, rejects, misfits or seconds are not acceptable for use on irrigation facilities.

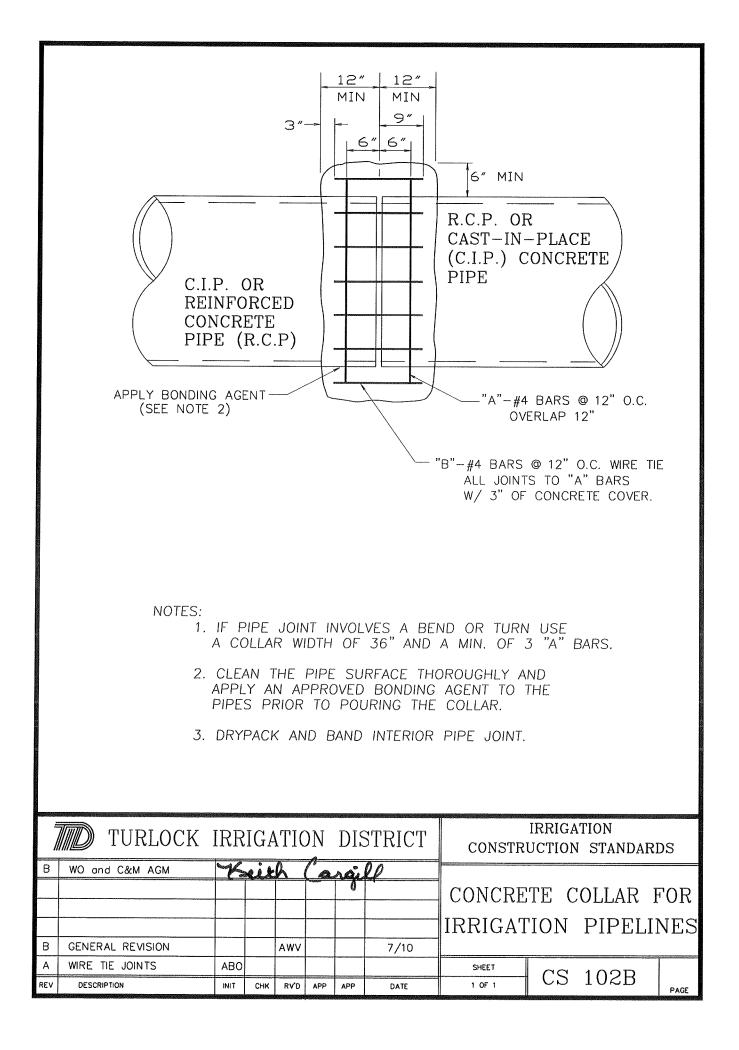
IRRIGATION STANDARDS	TURLOCK IRRIGATION DISTRICT	CONSTRUCTION AND DEVELOPMENT STANDARD			
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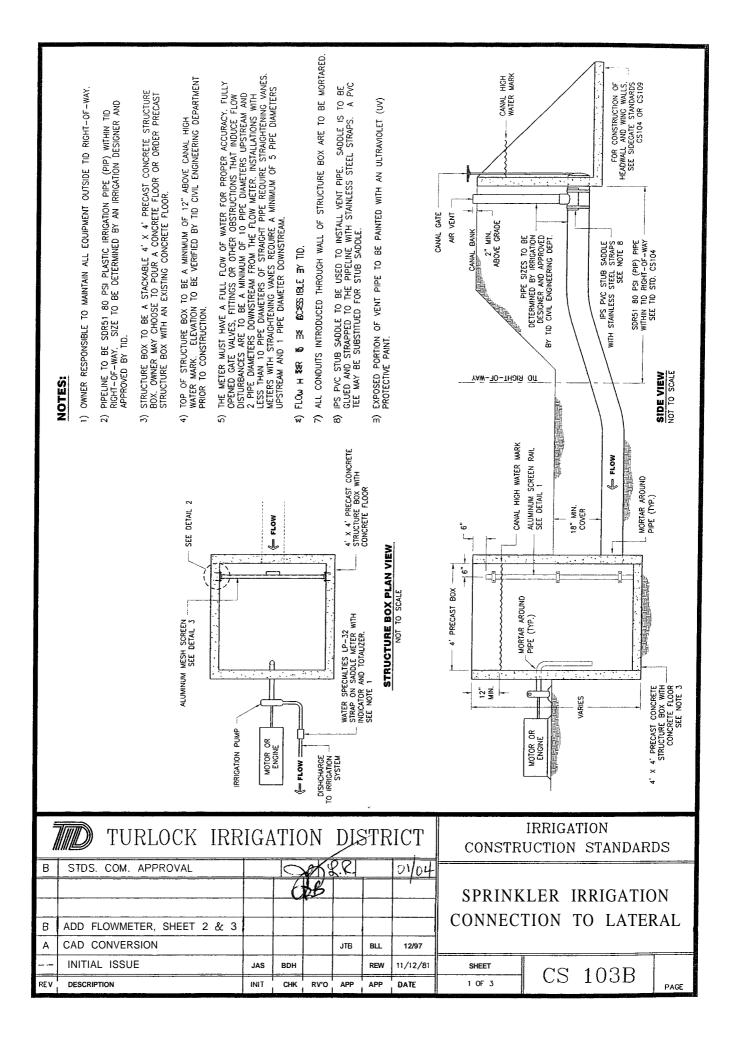


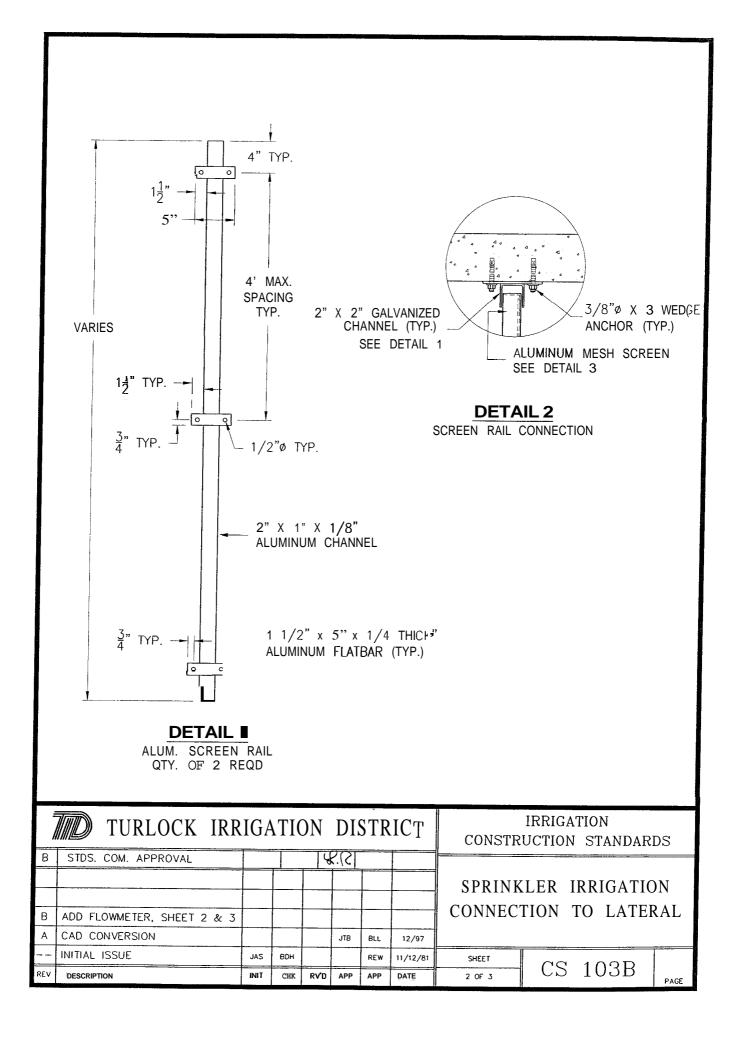


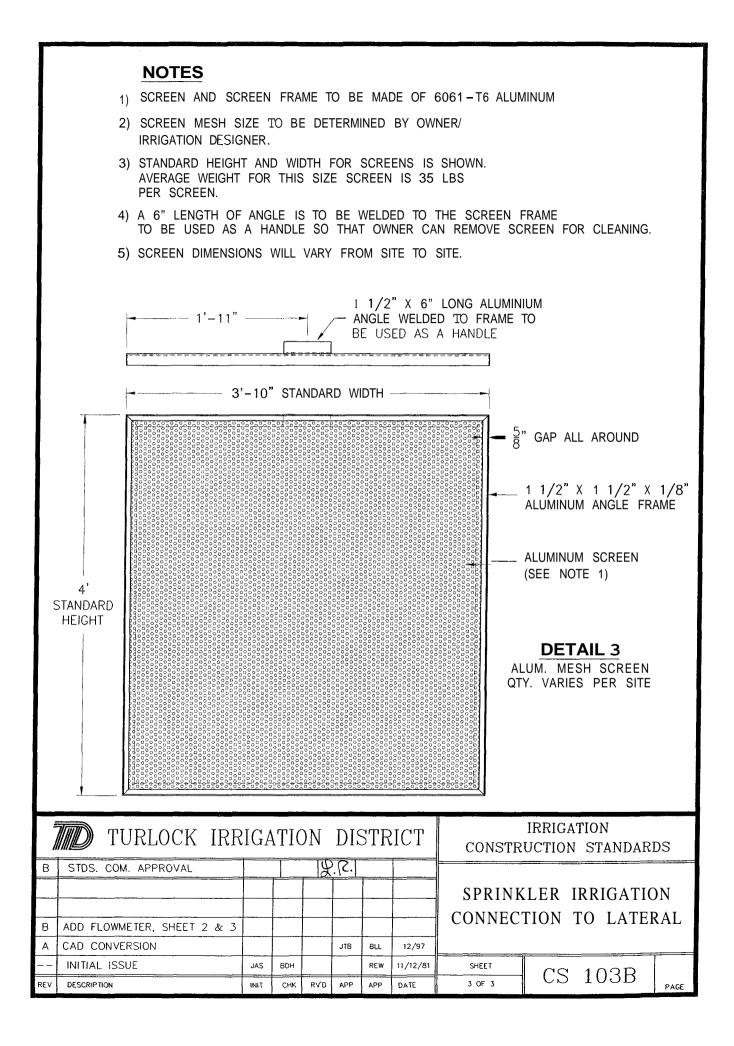


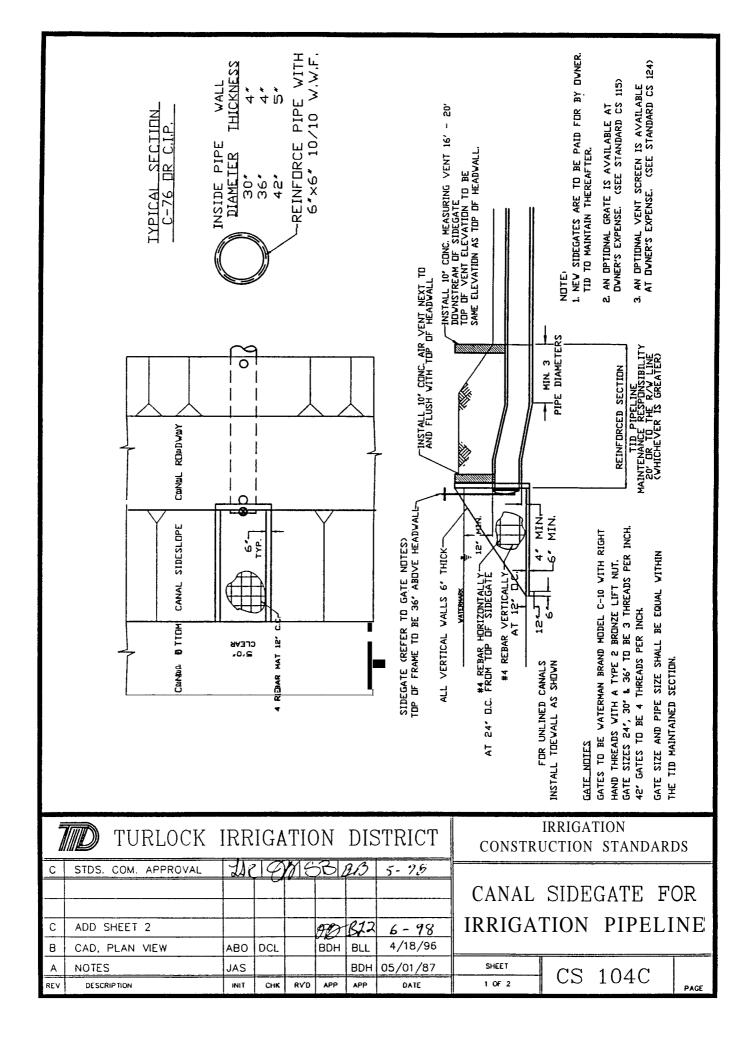


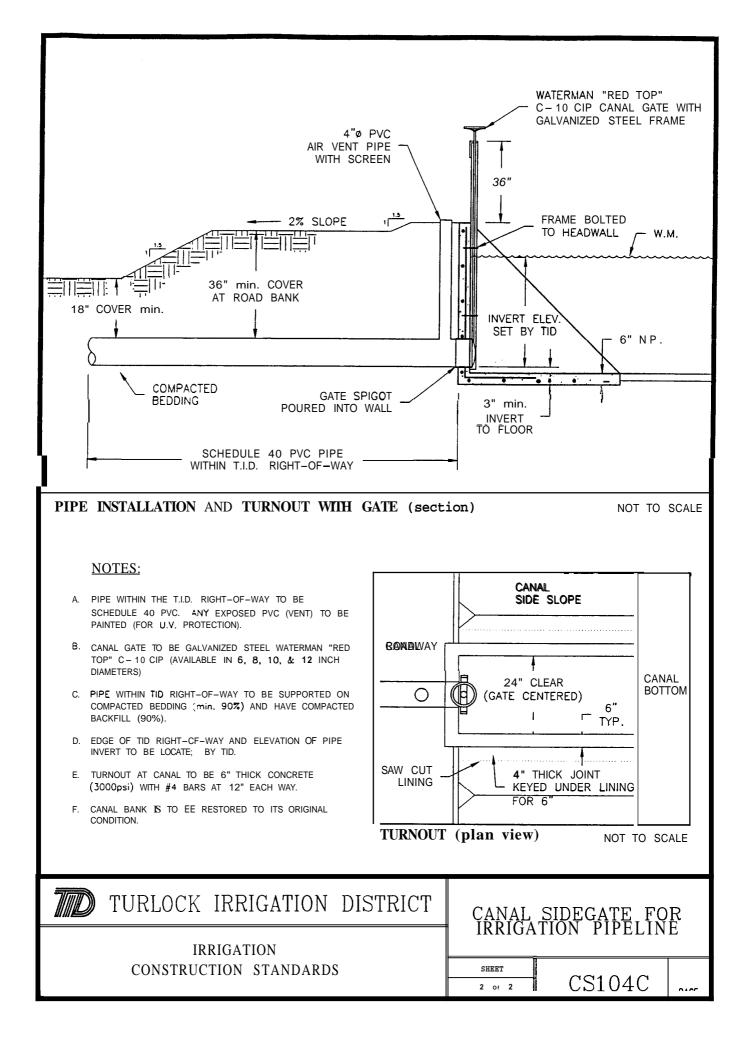


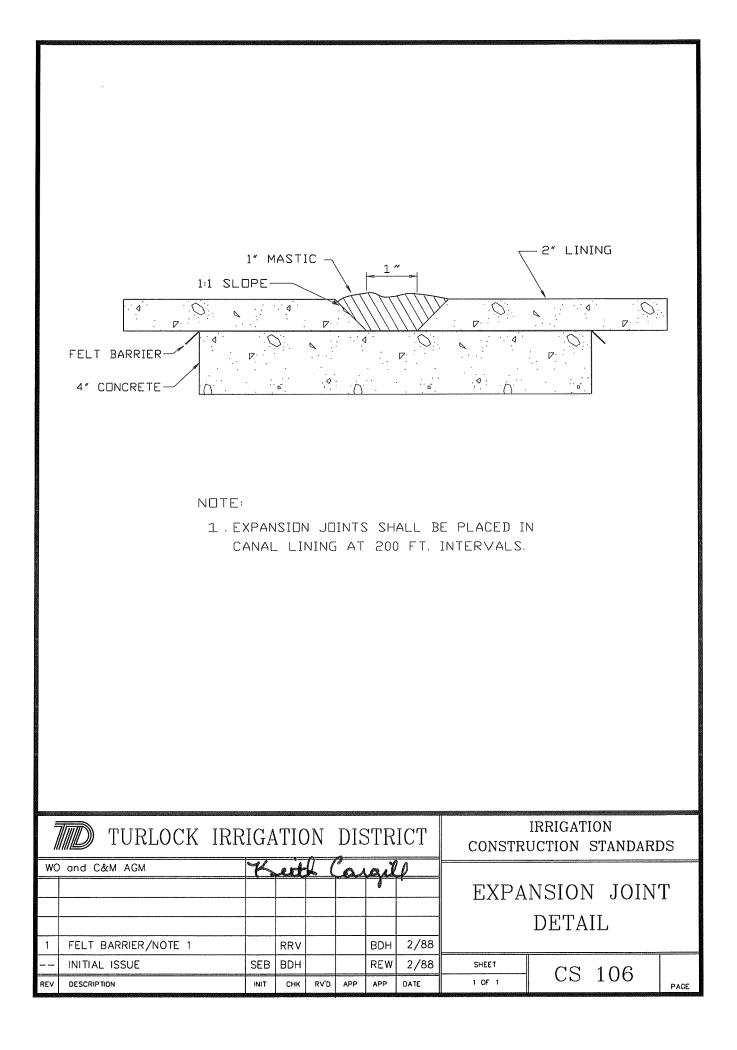


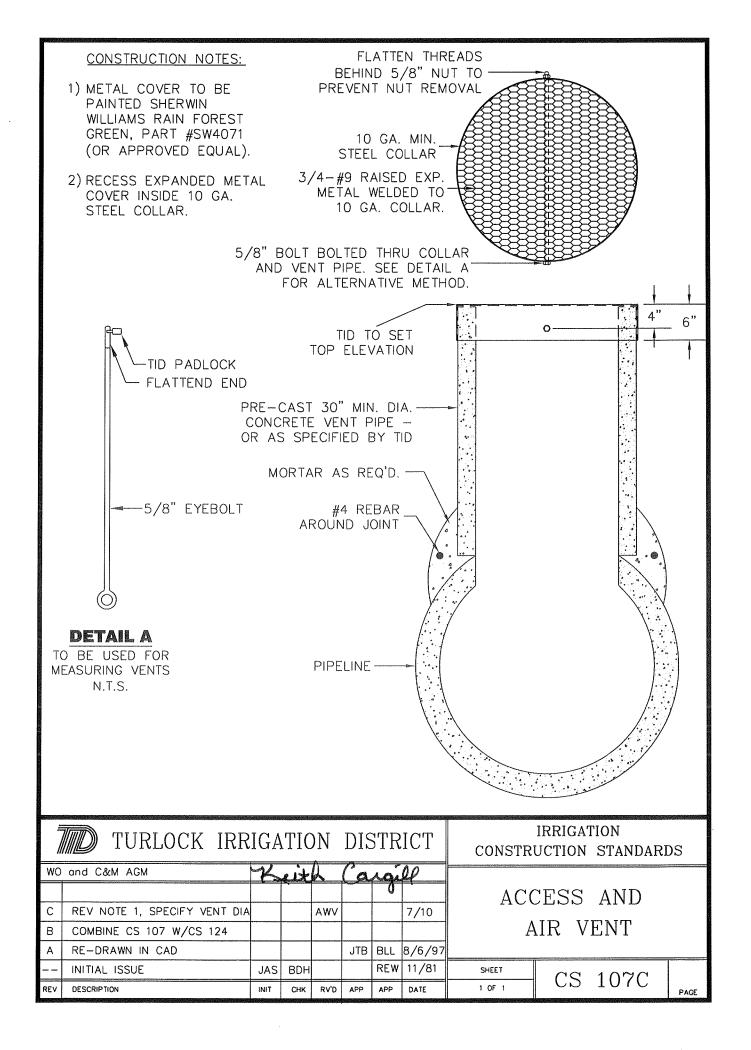


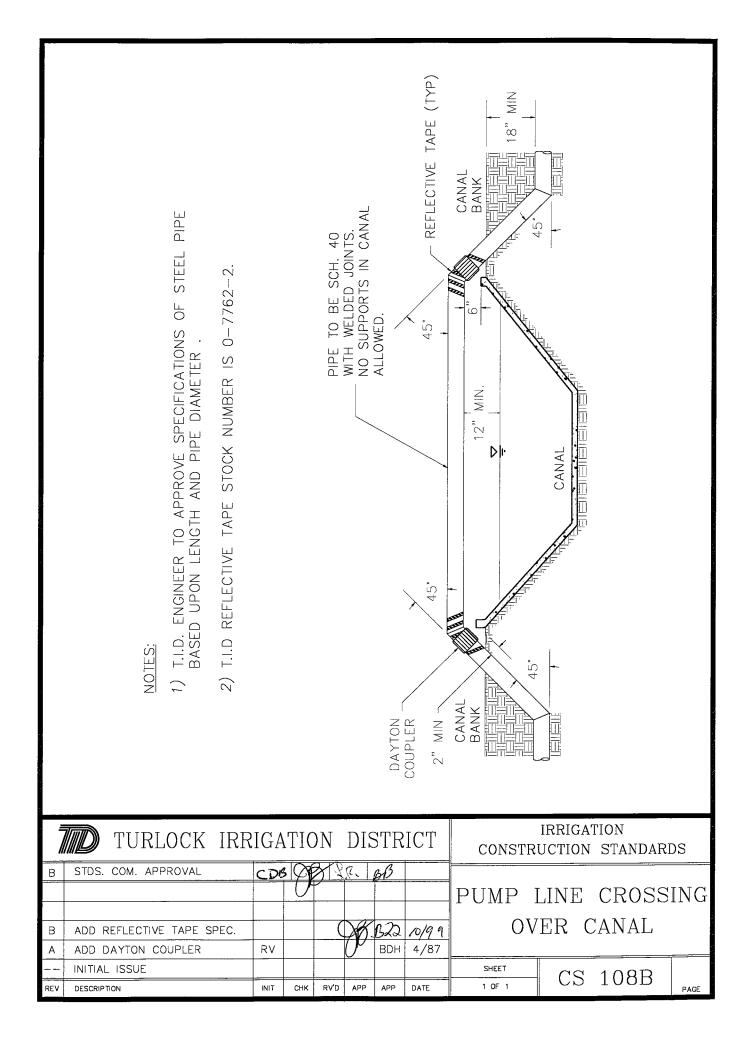


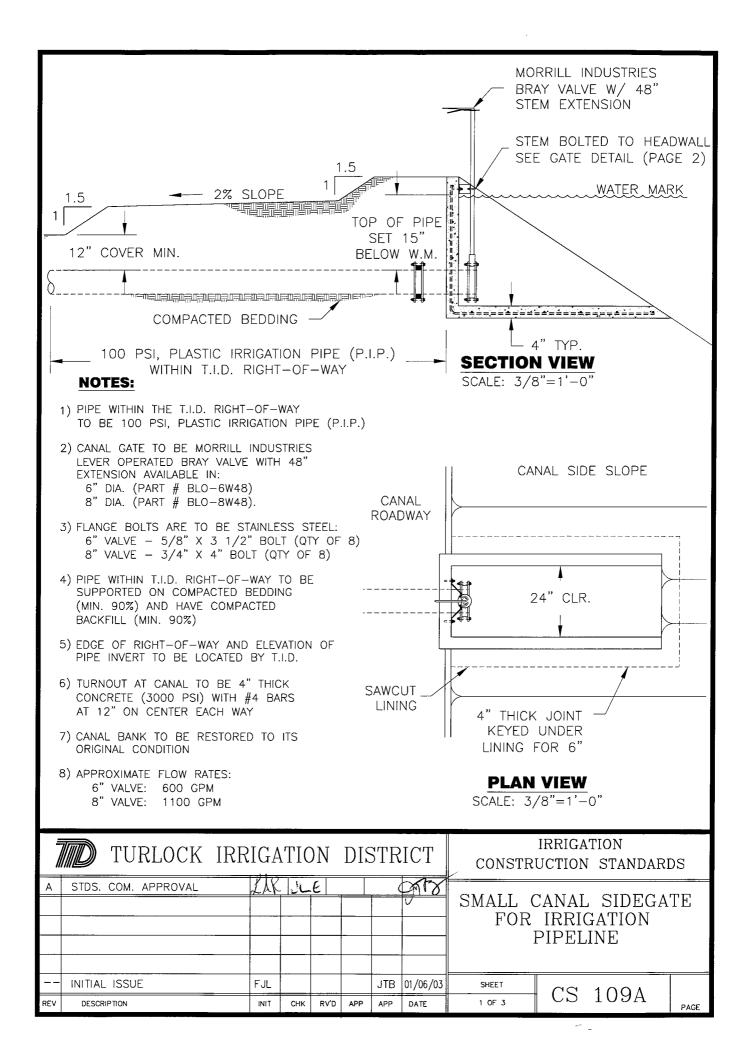


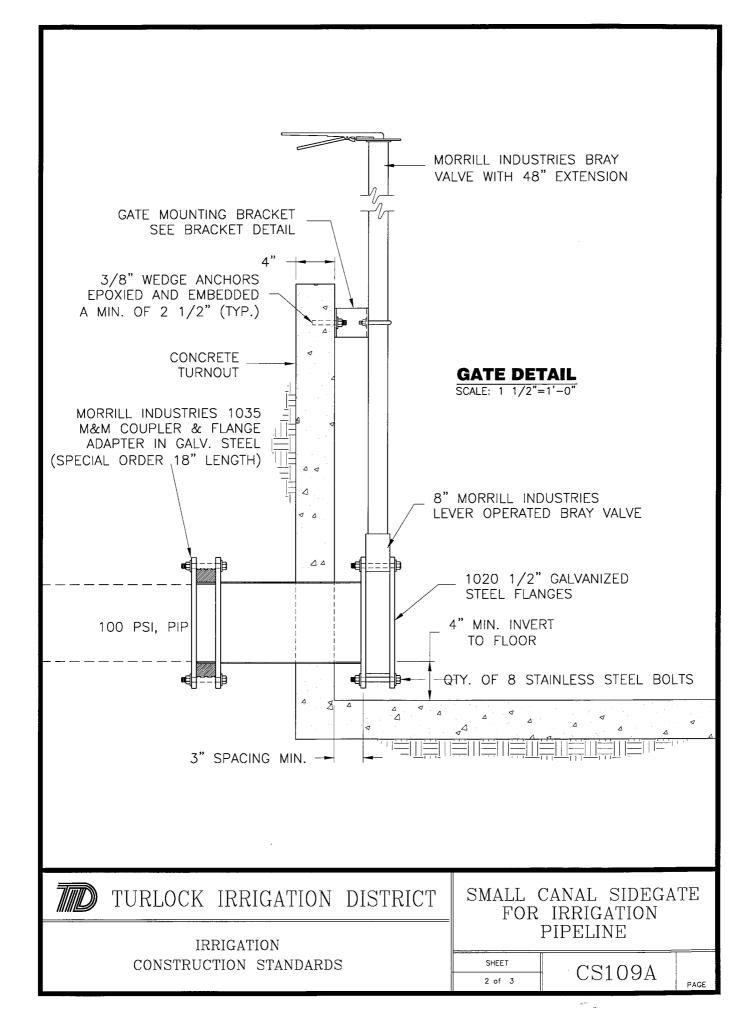


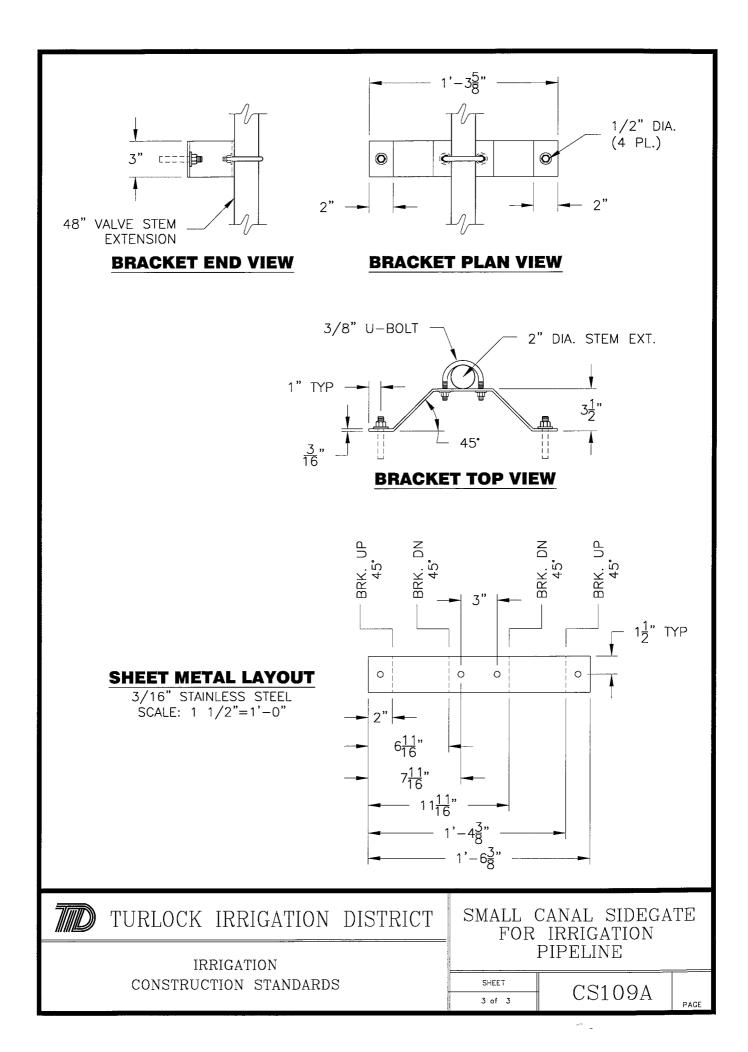


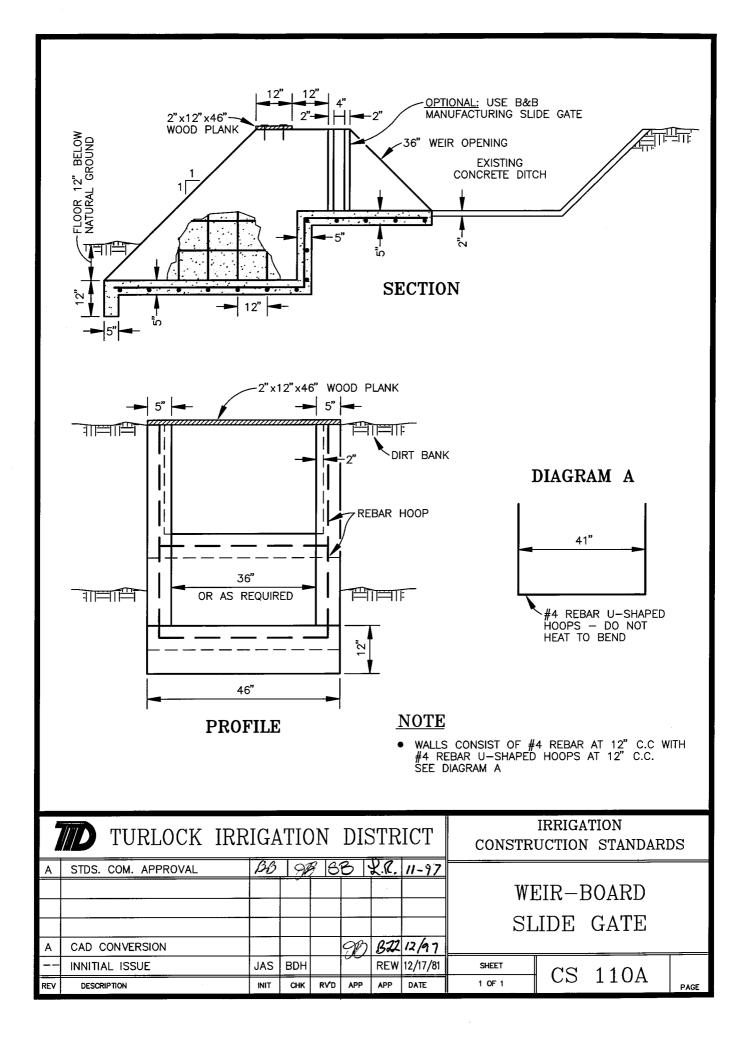




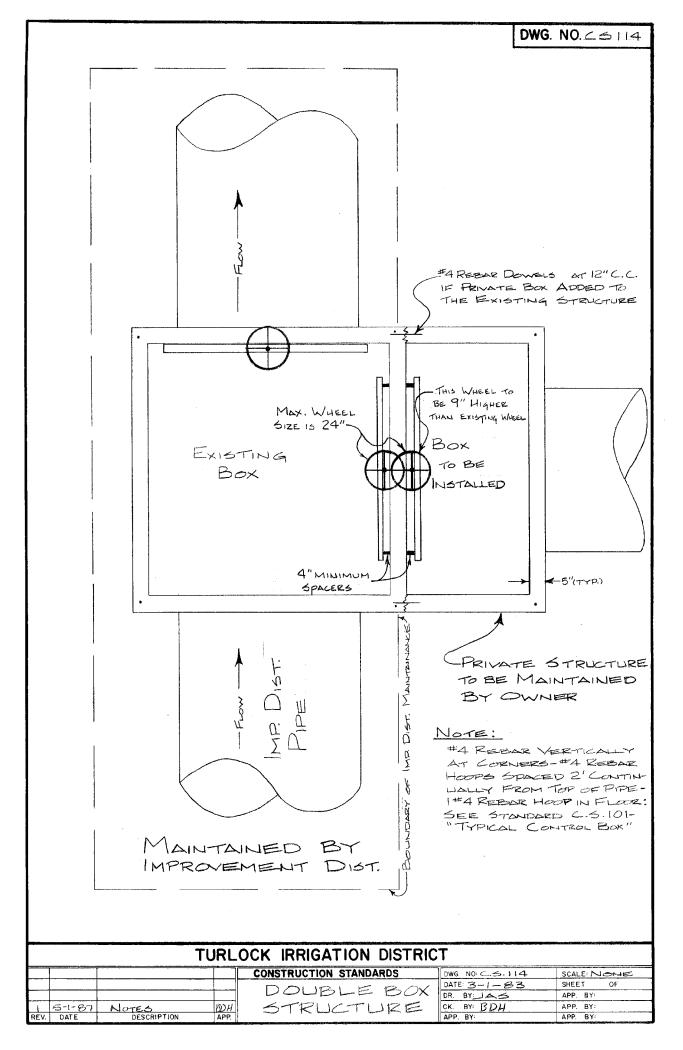


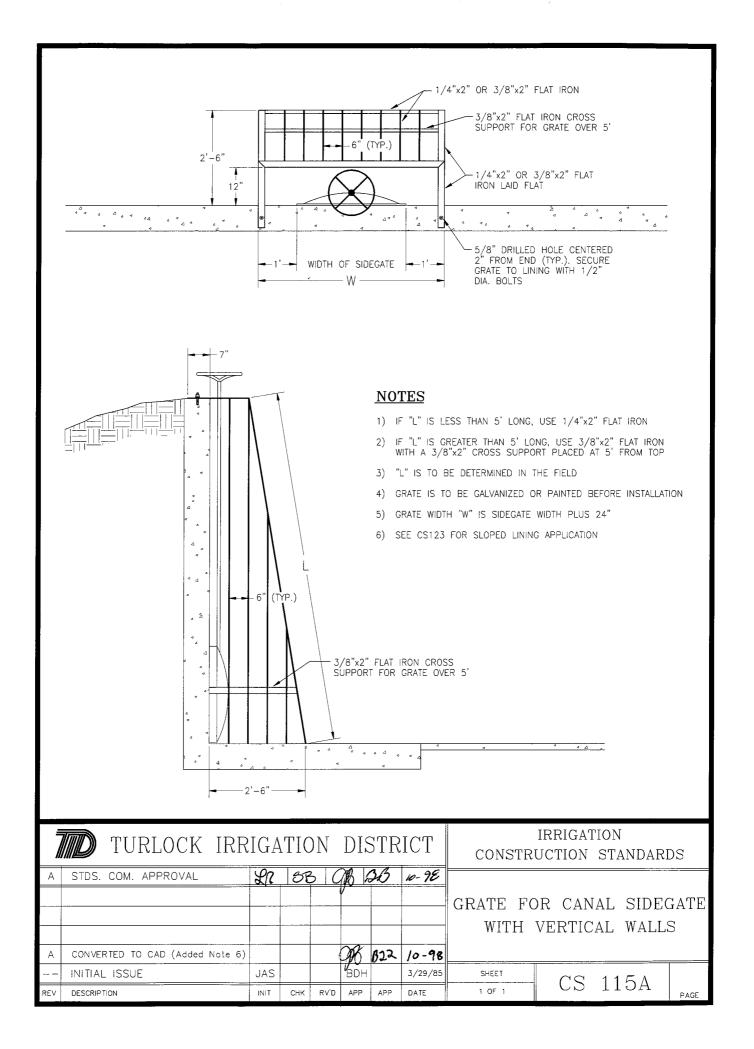


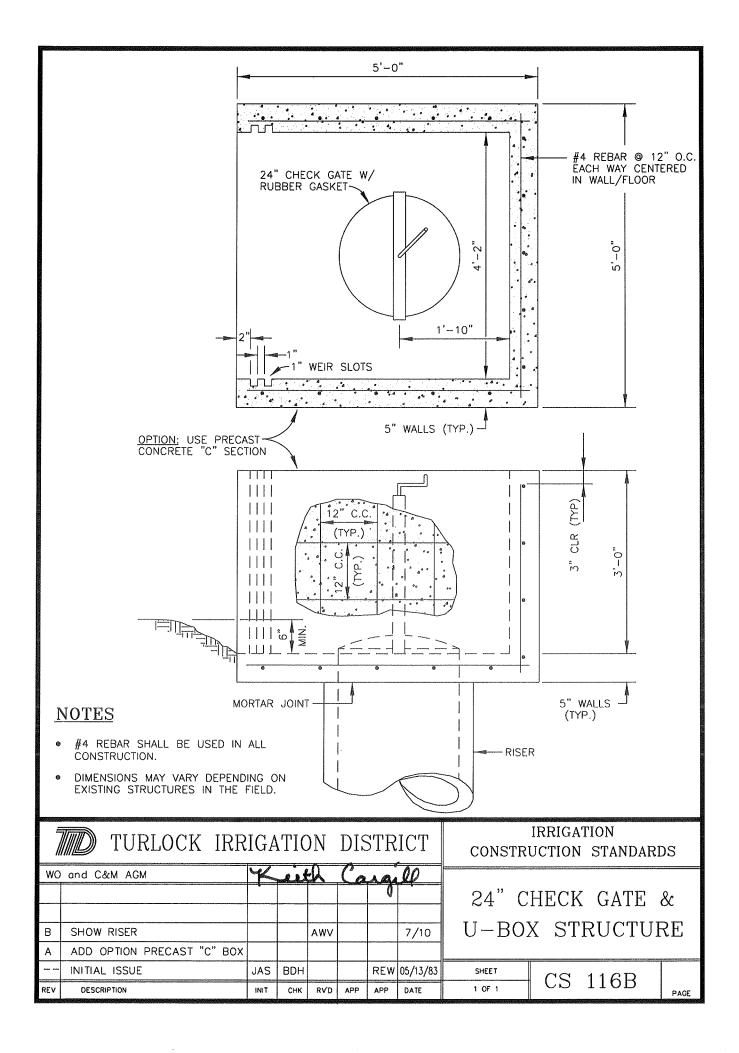


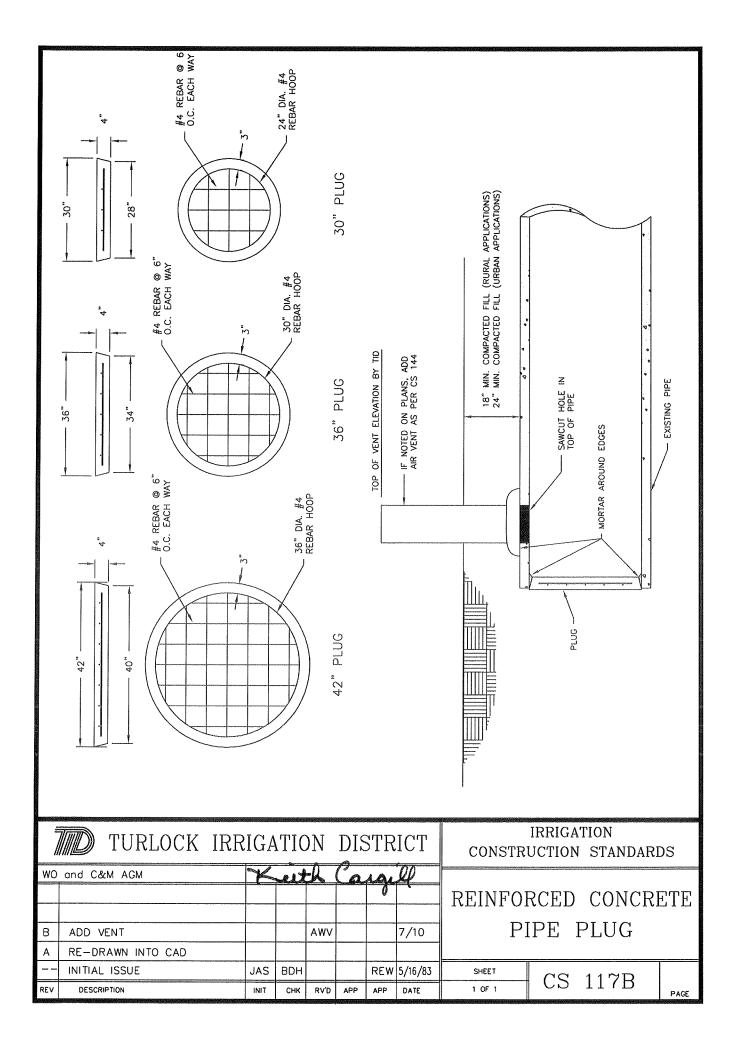


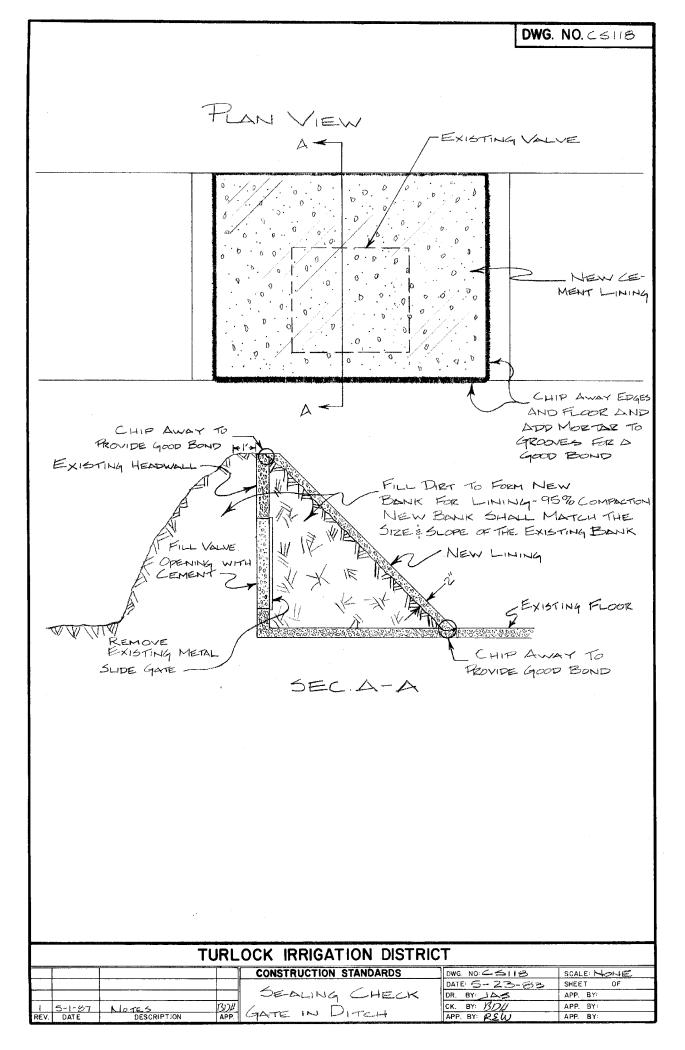
OVED EQU, RGER VENLY SPA SPACSD. DLOCK WIT TLLIAMS R/ ILLIAMS R/ TLLIAMS R/	REFLECTIVE TAPE	3' TID TID TID 1 MIN POST HOLE SHALL BE A MINIMUM 0F 12" DIA. USING CONCRETE TO 0F 12" DIA. USING CONCRETE TO ANCHOR POST IN HOLE.
TURLOCK IRRIGATION		RIGATION FION STANDARDS
D STDS. COM. APPROVAL	METAL	GATE FOR
CADDED NOTE 2/ADDED SPEC. 1JBCHANGED SPEC. 4, 5, & 9JB	LR 208 10/01/02	L ROADS
B ADDED NOTE/CHANGED COLOR A ADDED SPECIFICATION 7 & 8 SEB	JB BLL 5/11/99 4/22/87 SHEET 6	20 112D
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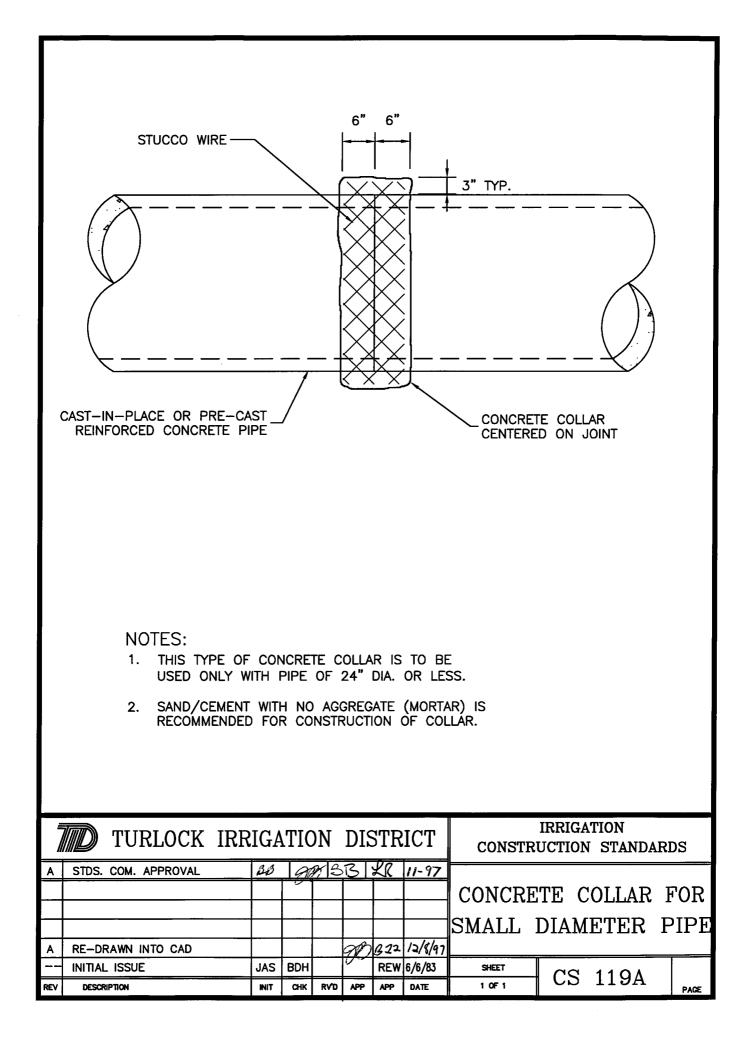


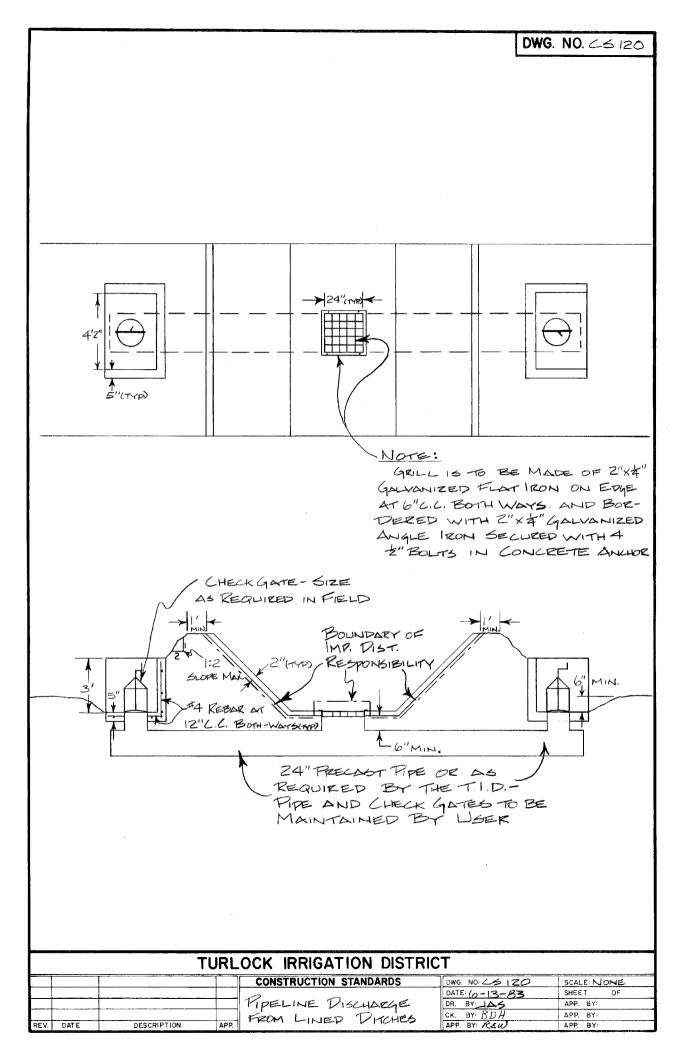


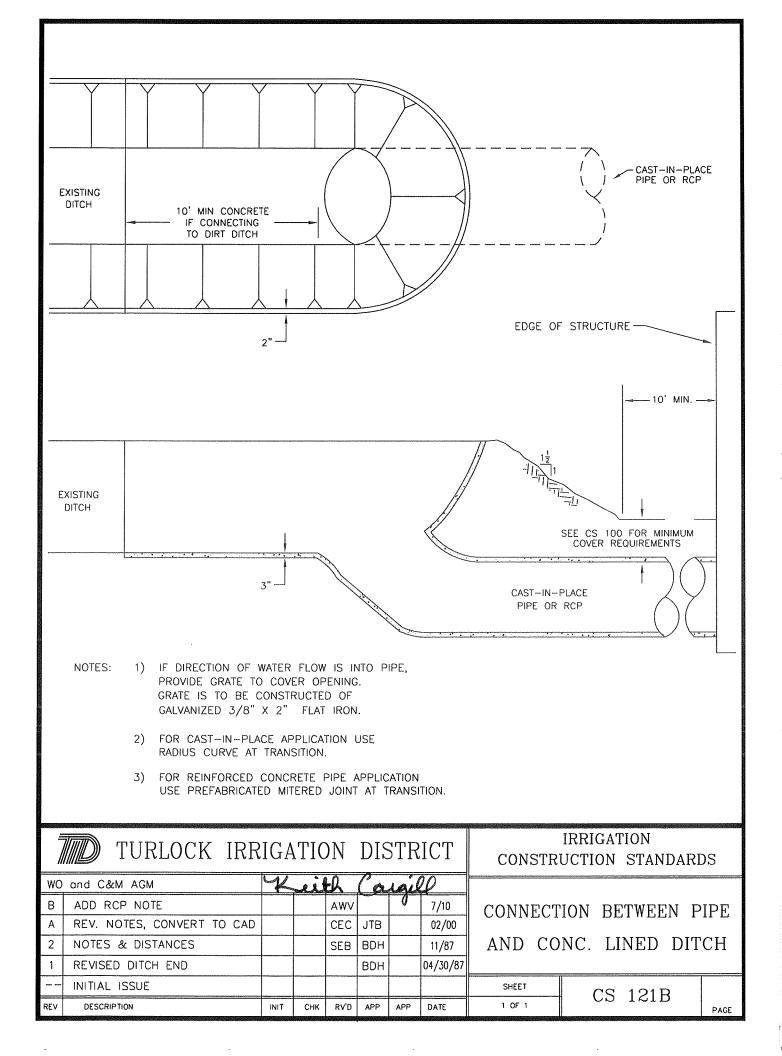


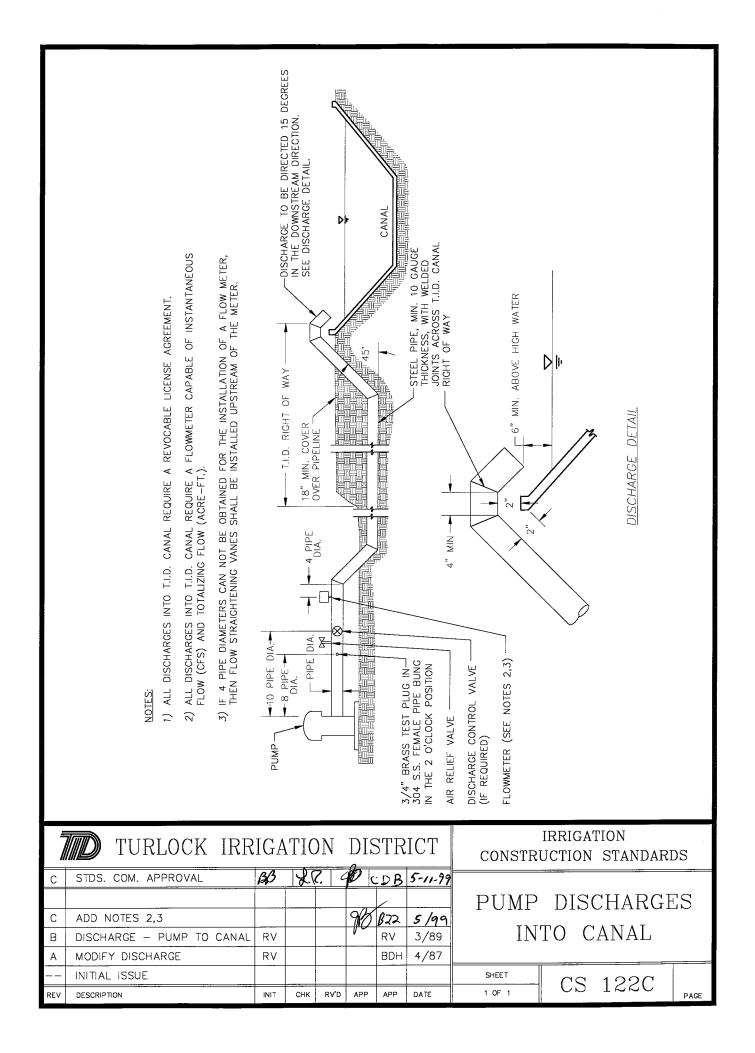


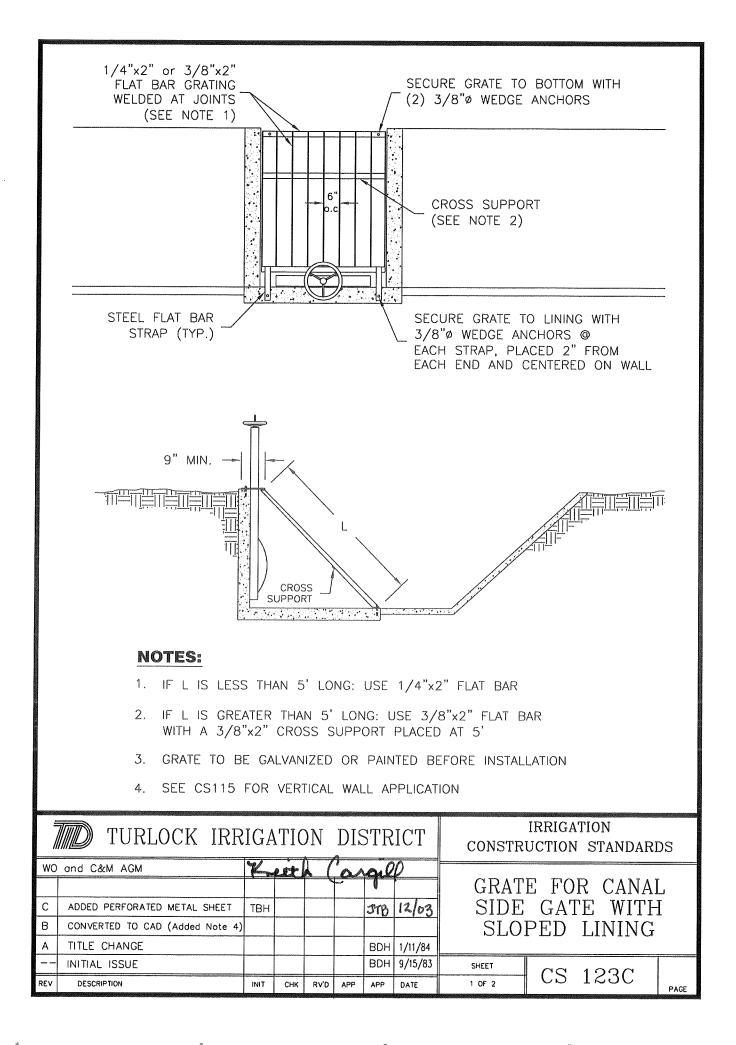


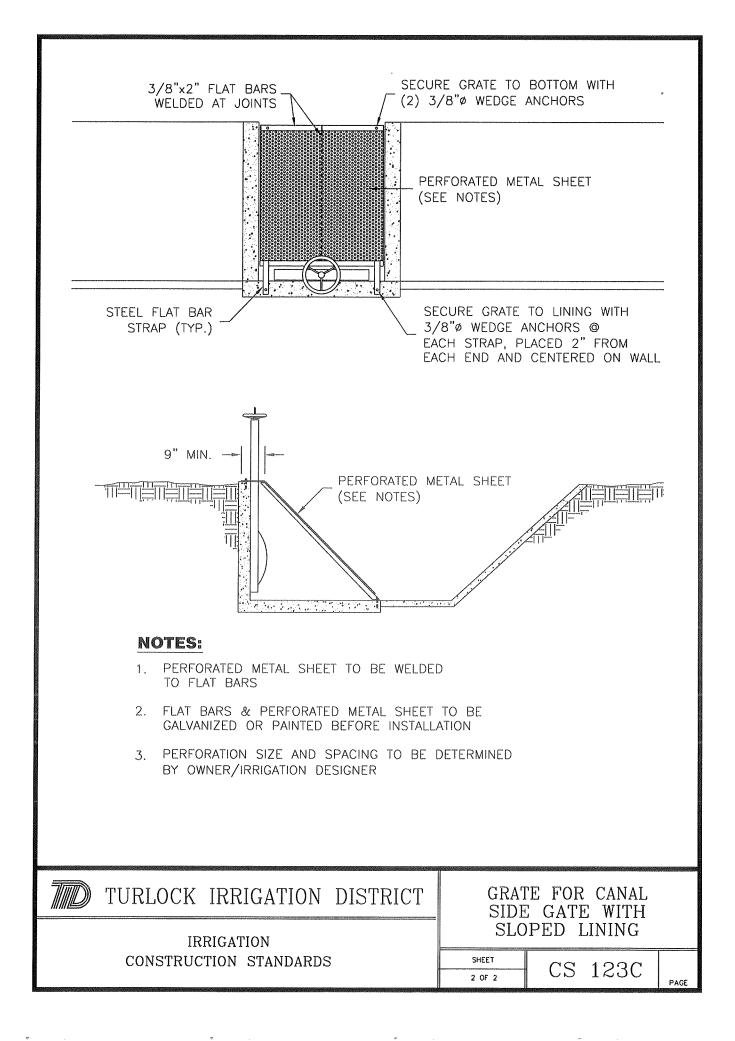


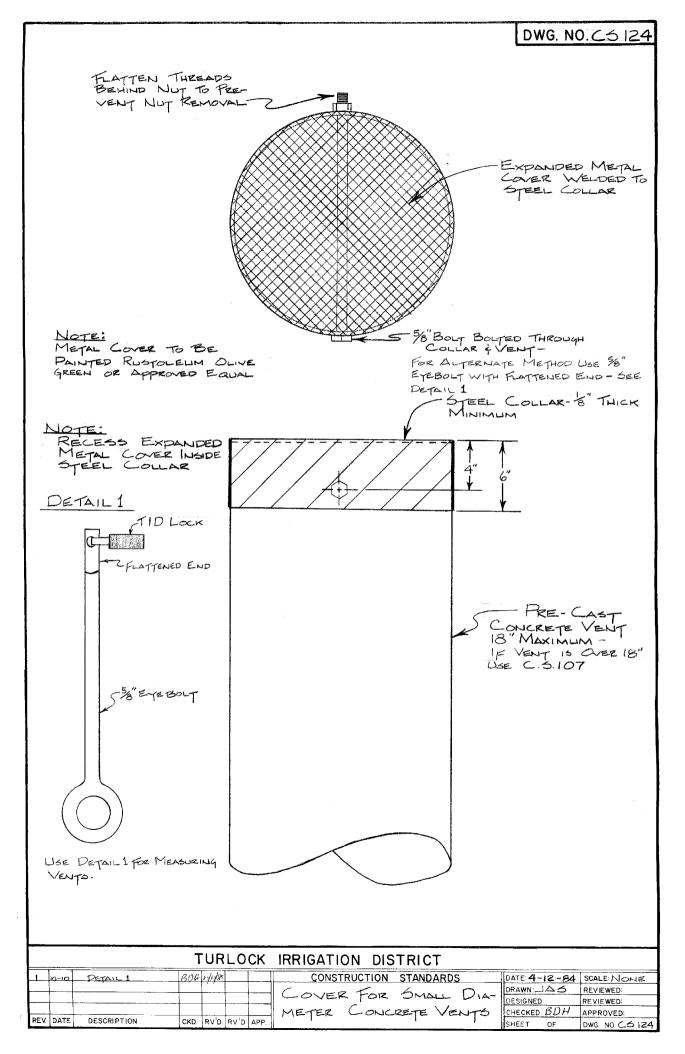






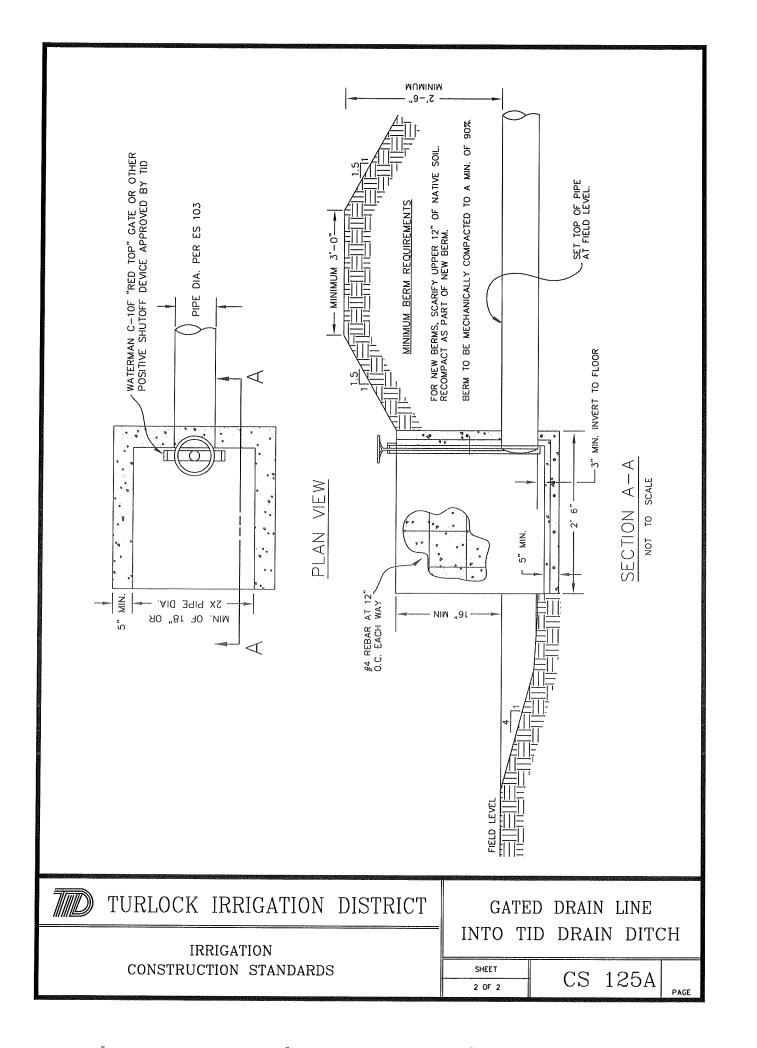


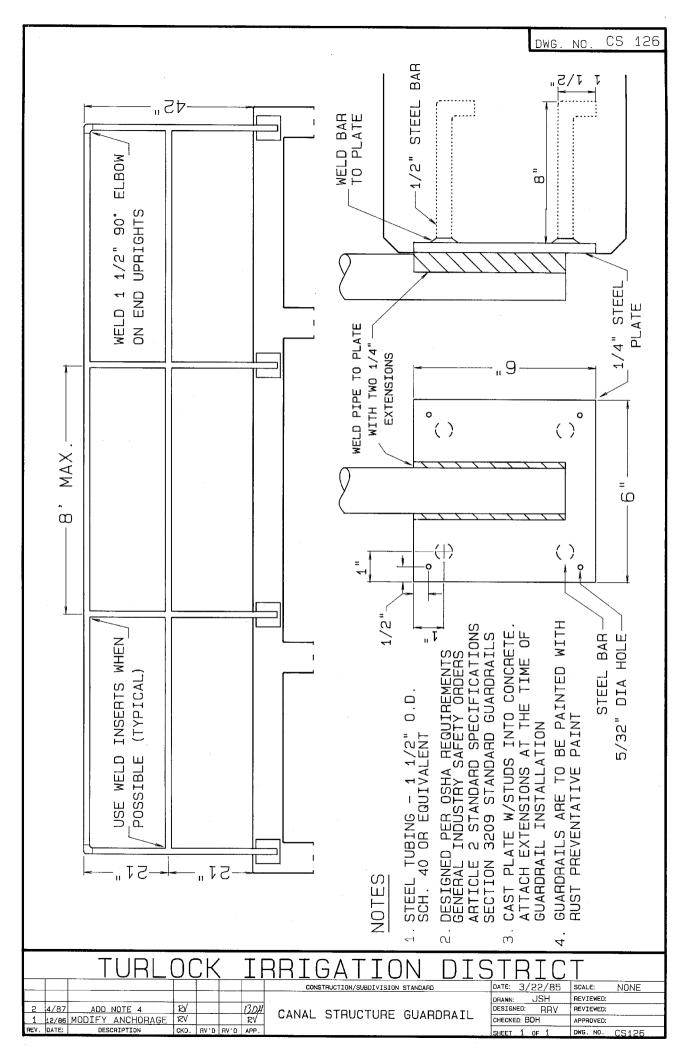


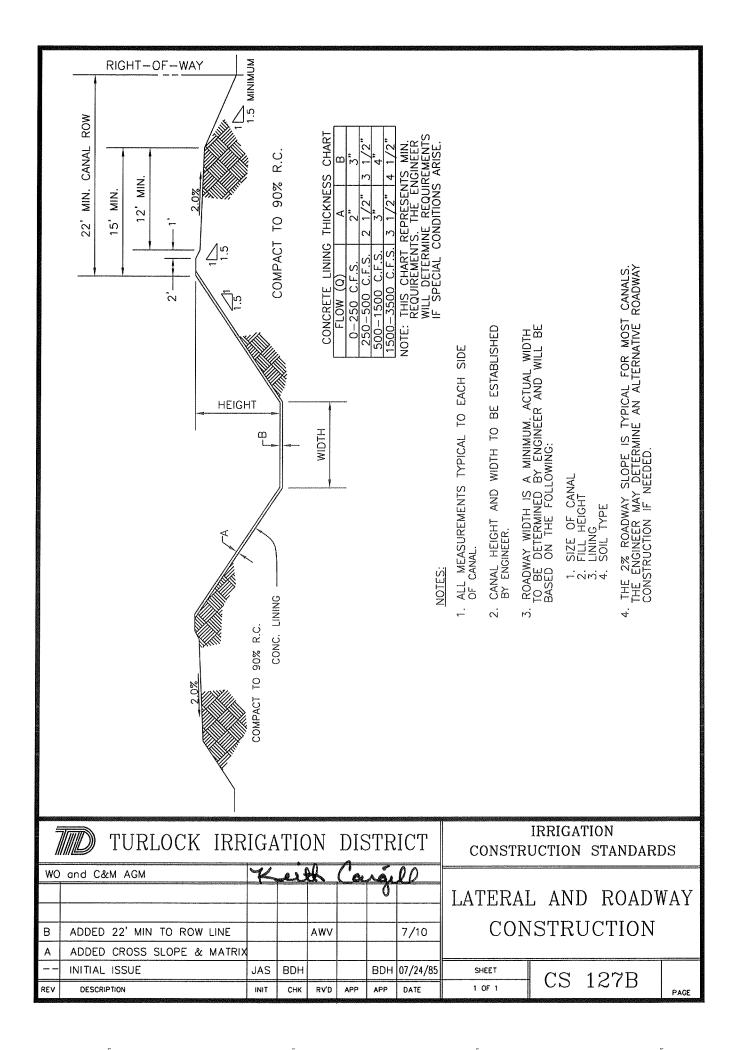


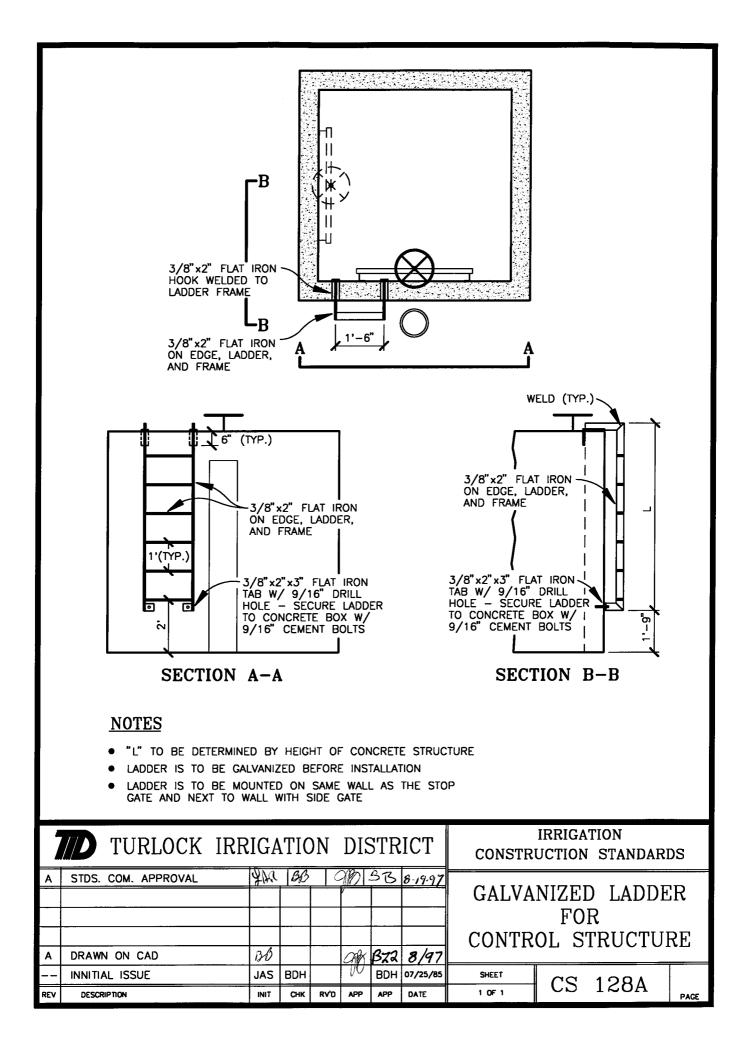
MET MINIMUM BANK REQUIREMENTS OR MAINTAIN EXISTING EMBANKMENT HEIGHT WHICHEVER IS GREATER DRAIN FLAP GATE WATERMAN F-10SB OR APPROVED EQUAL	AILER LEVEL	ION	IF PIPE EXTENDS MORE THAN 1/3 OF ITS LENGTH INTO DRAIN, SUPPORT END WITH PRESSURE TREATED OR REDWOOD SUPPORT.	CONCRETE 3-WALL STRUCTURE SHALL CONSIST OF 5" WALLS WITH #4 REBAR AT 12" C.C. BOTH WAYS.	STRUCTURE BOX SHALL BE A MINIMUM OF 18" OR 2 TIMES THE PIPE DIAMETER.	BOTTOM OF PIPE MUST BE INSTALLED A MIN, OF 3" ABOVE THE INSIDE FLOOR OF STRUCTURE.	TOP OF PIPE AND FIELD LEVEL SHOULD BE THE SAME.	CANAL BANK IS TO BE RESTORED TO ITS ORIGINAL CONDITION. MAINTAIN EMBANKMENT HEIGHT.	DRAINS CROSSING PUBLIC ROADS MUST MEET ALL CITY/COUNTY REQUIREMENTS AND SPECIFICATIONS.
UM BAN IS GRE		SECTION	7)	8)	6	10)	11)	12)	13)
MEET MINIMUM MAINTAIN EXIST WHICHEVER IS O	SHEET 2 OF 2	NOTES: CROSS SE	 NOTIFY TID 24 HOURS BEFORE STARTING WORK. THE INSTALLATION MUST BE INSPECTED BEFORE IT IS BACKFILLED. 	2) DRAINS MUST BE AT LEAST 200 FEET FROM CATTLE PENS. NO MANURE MAY BE DISCHARGED INTO THE DRAIN AT ANY TIME.	mU	16 GAUGE CALVAN	2	5) END OF PIPE MUST EXTEND PAST THE EDGE OF THE MINIMUM POOL.	6) WATERMAN RED TOP FLAP GATE F-10SB OR APPROVED EQUAL MUST BE USED IF PIPE IS BELOW THE HIGH WATER LINE.
TURLOCK IRRIGATION DISTRICT IRRIGATION CONSTRUCTION STANDARDS									
WO and C&M AGM B REVISE NOTES A ADD GATE & MIN. BERM REQ.	AWV		7/10 L 4/00	IN	GATI TO '	ED TID		AIN RAIN	LINE DITCH
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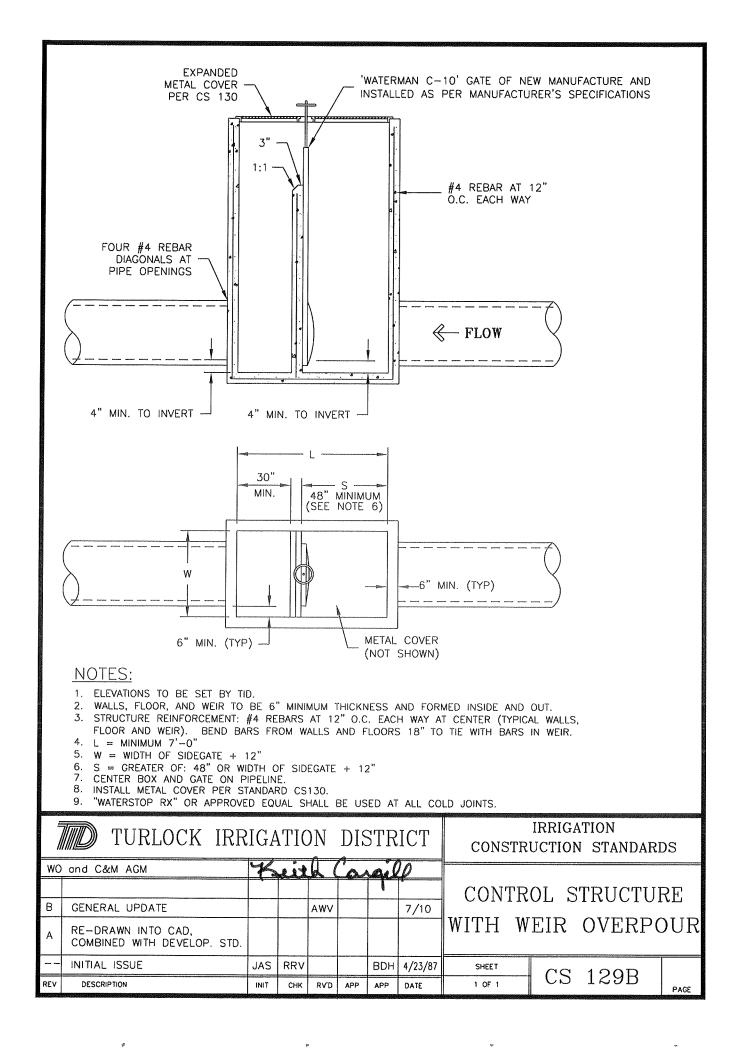
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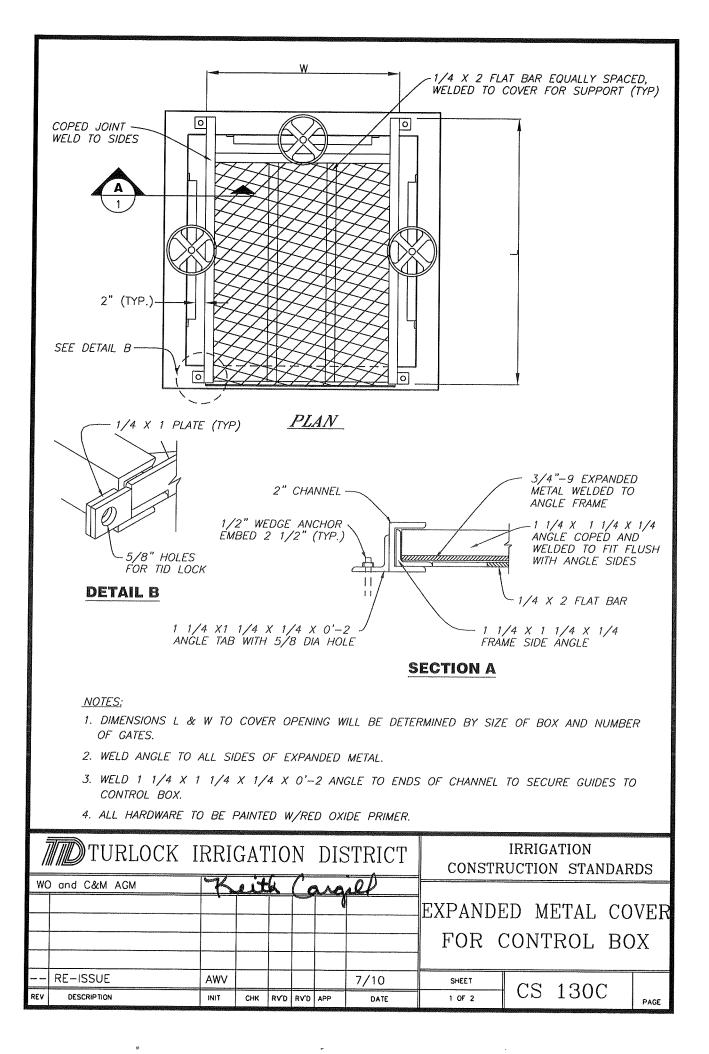


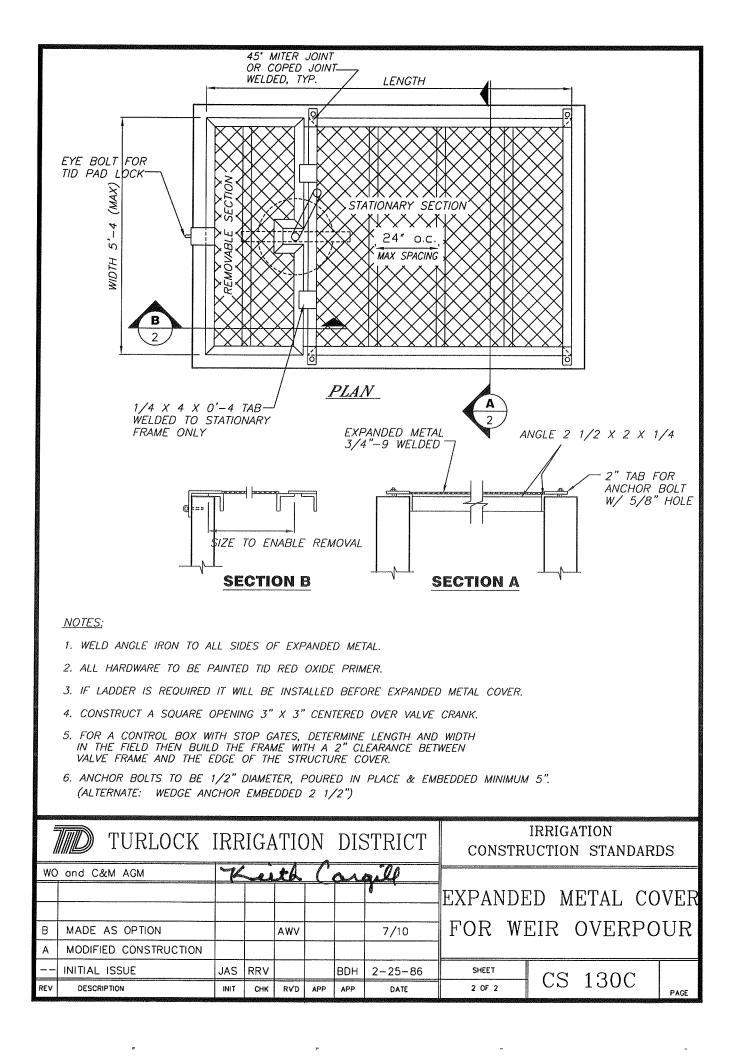


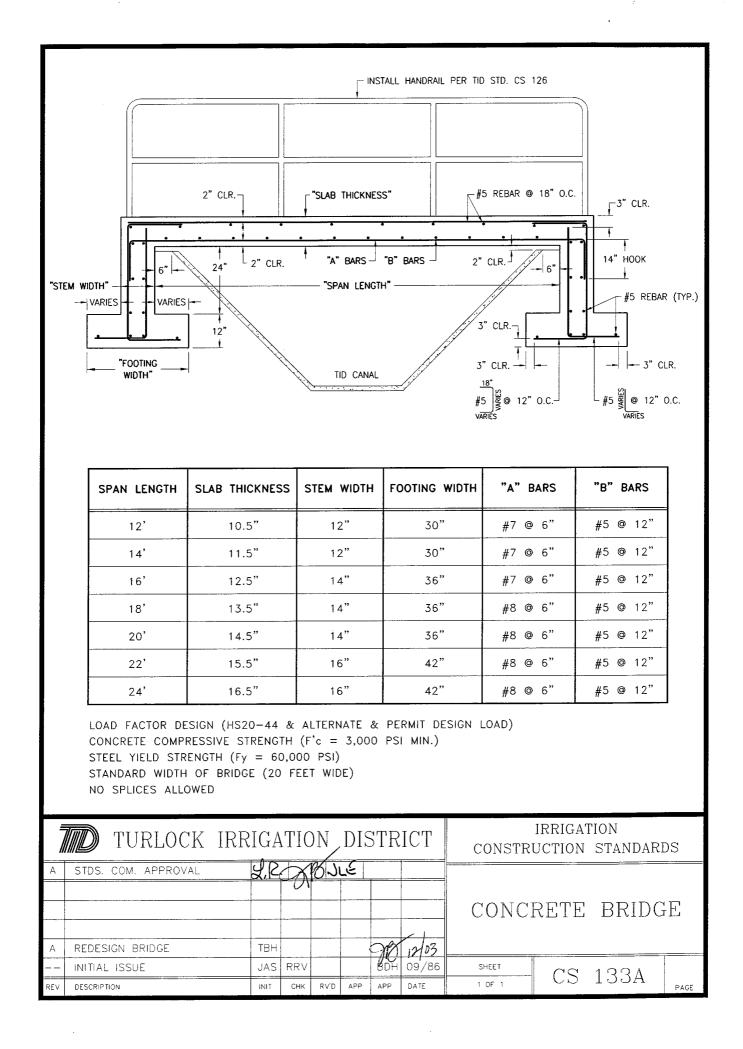


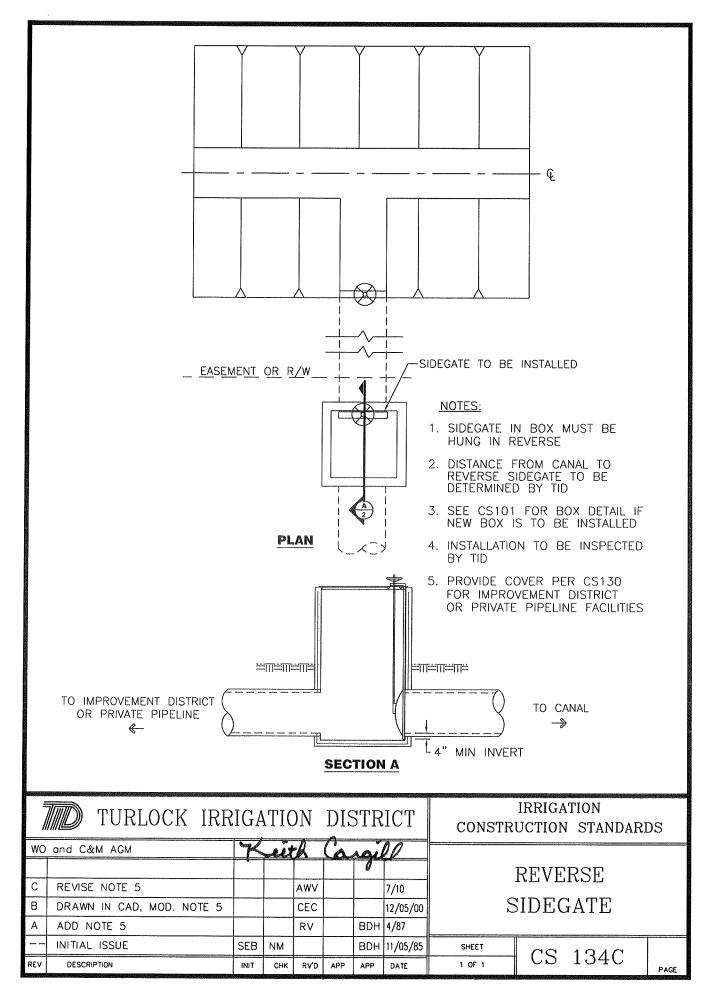




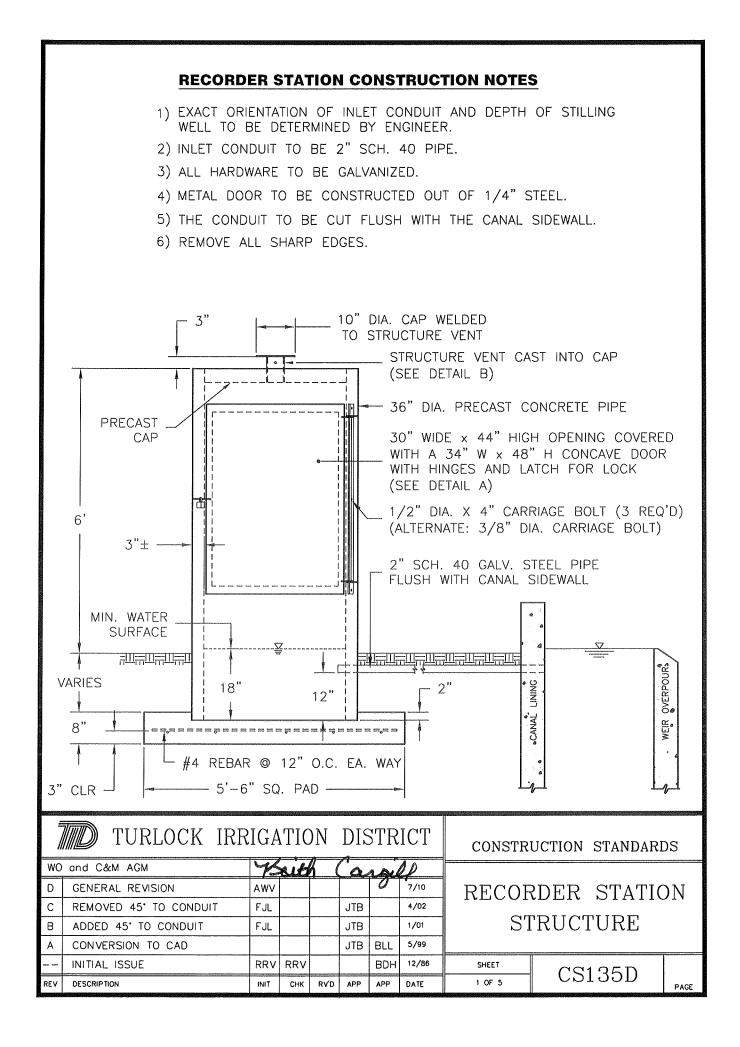


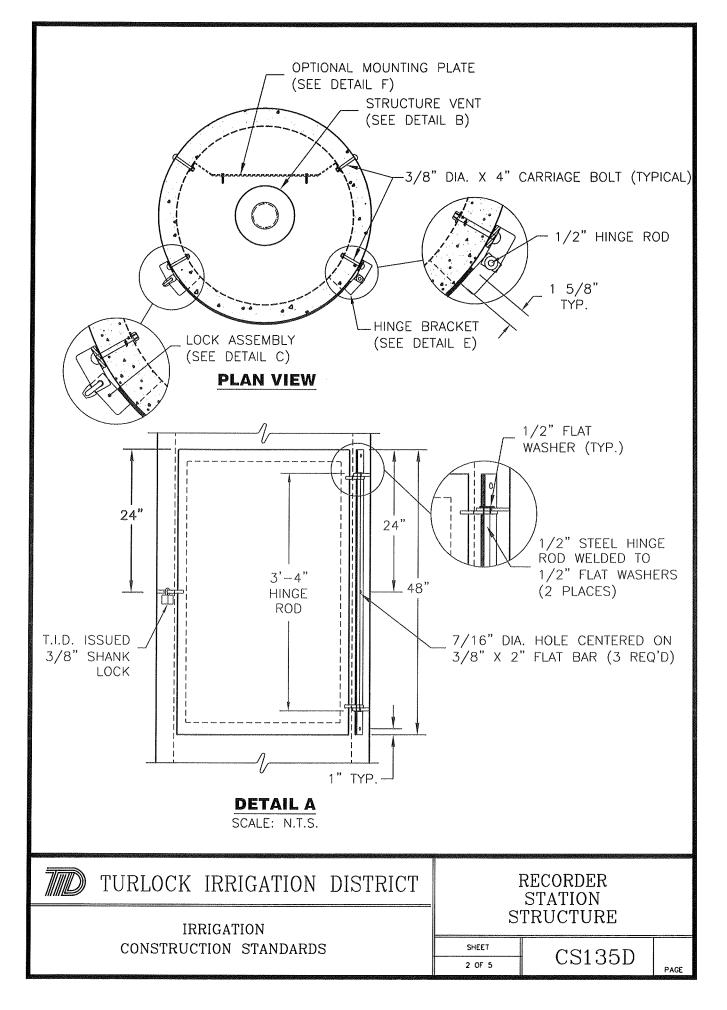






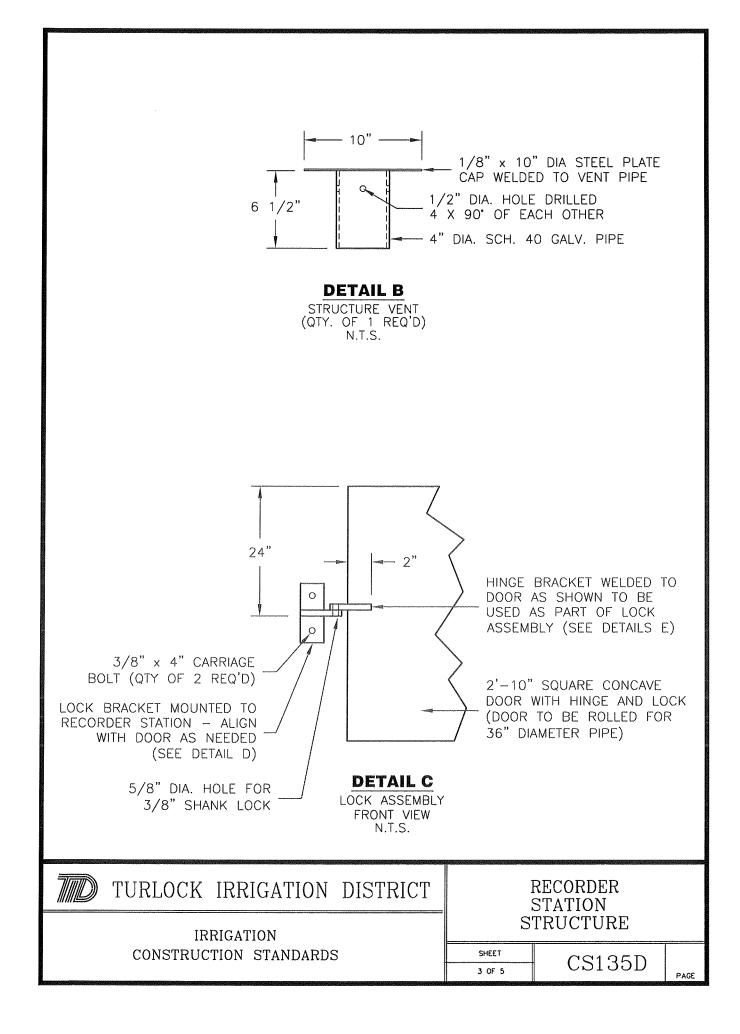
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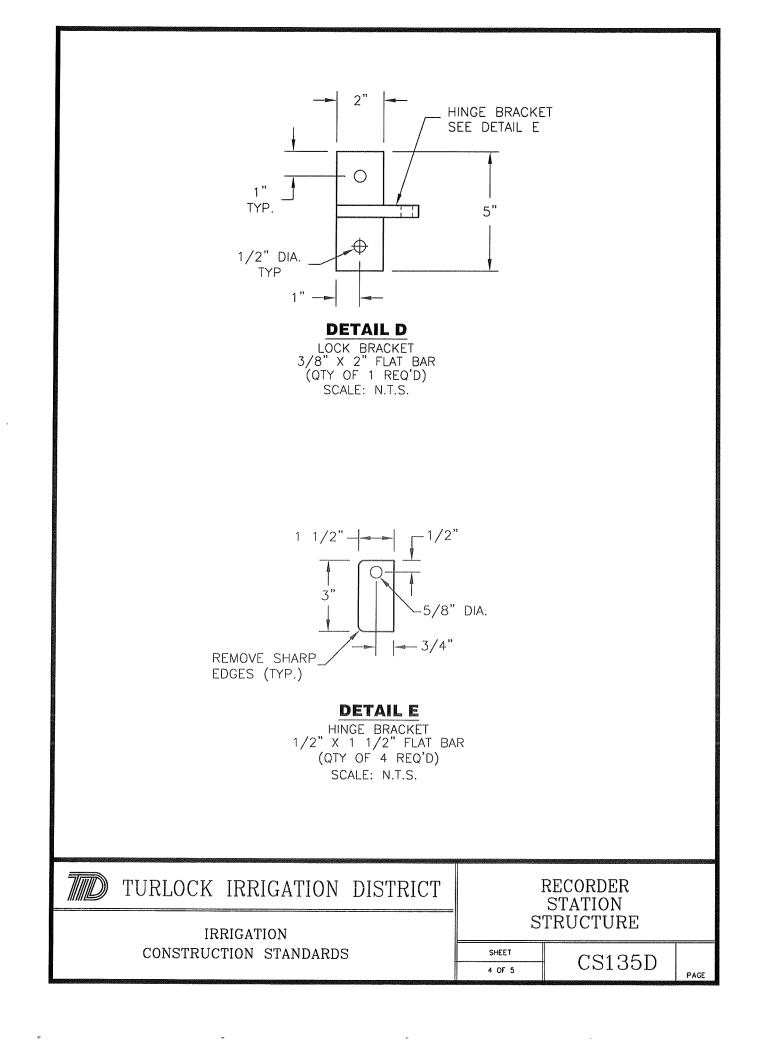




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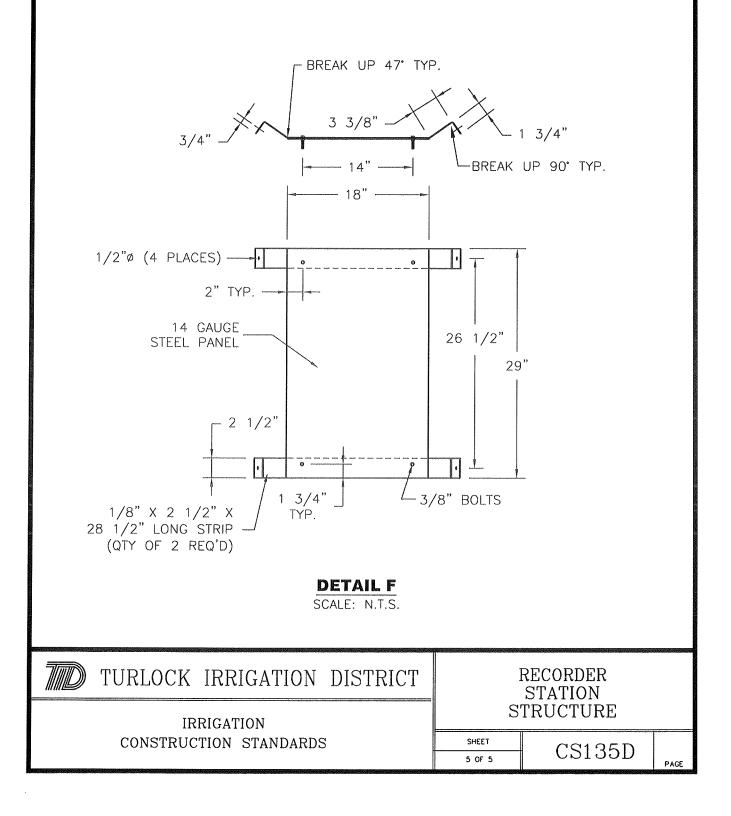


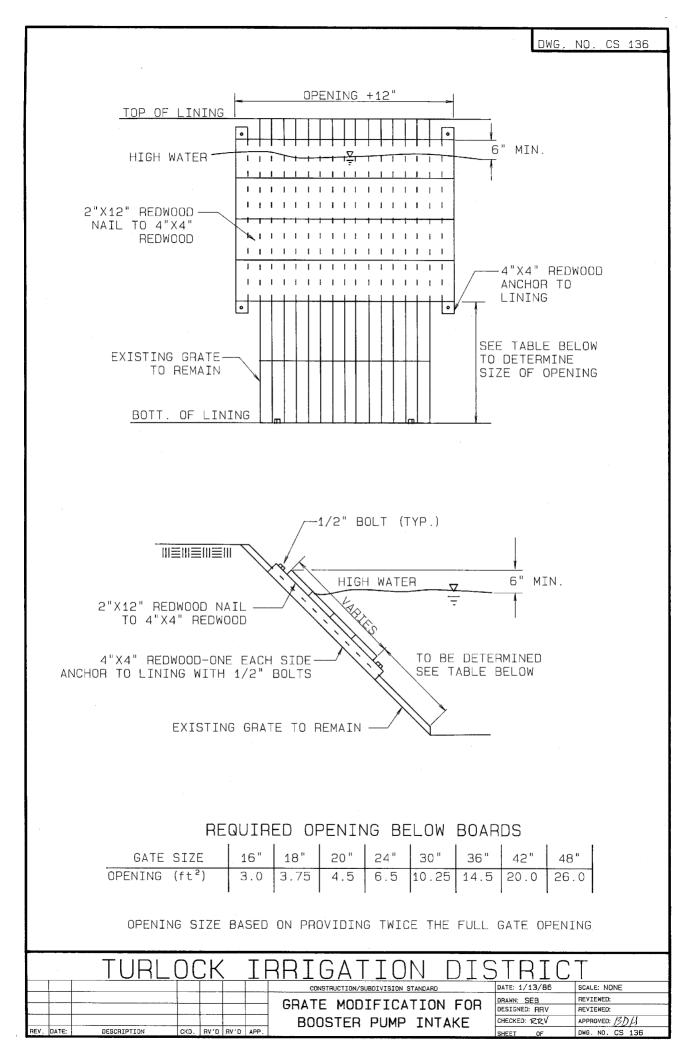


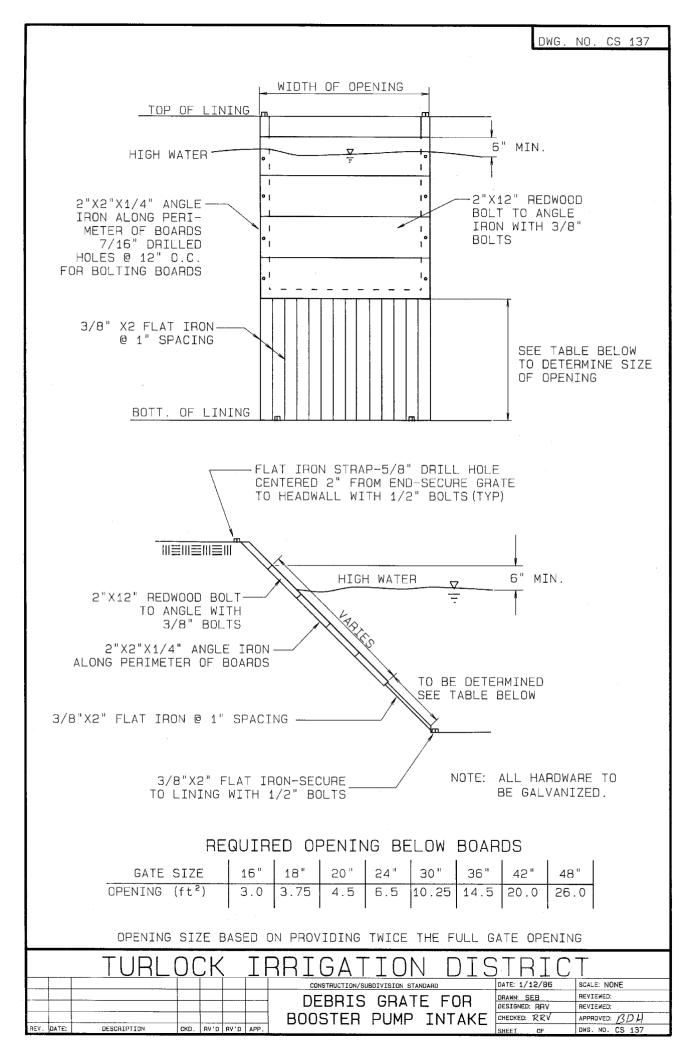
RECORDER STATION MOUNTING PLATE

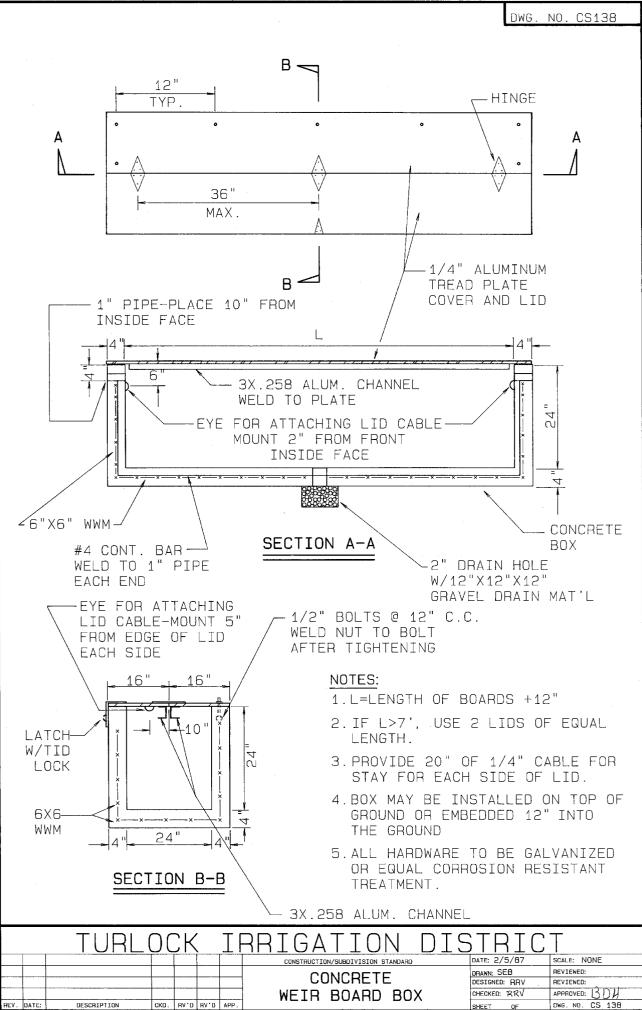
CONSTRUCTION NOTES:

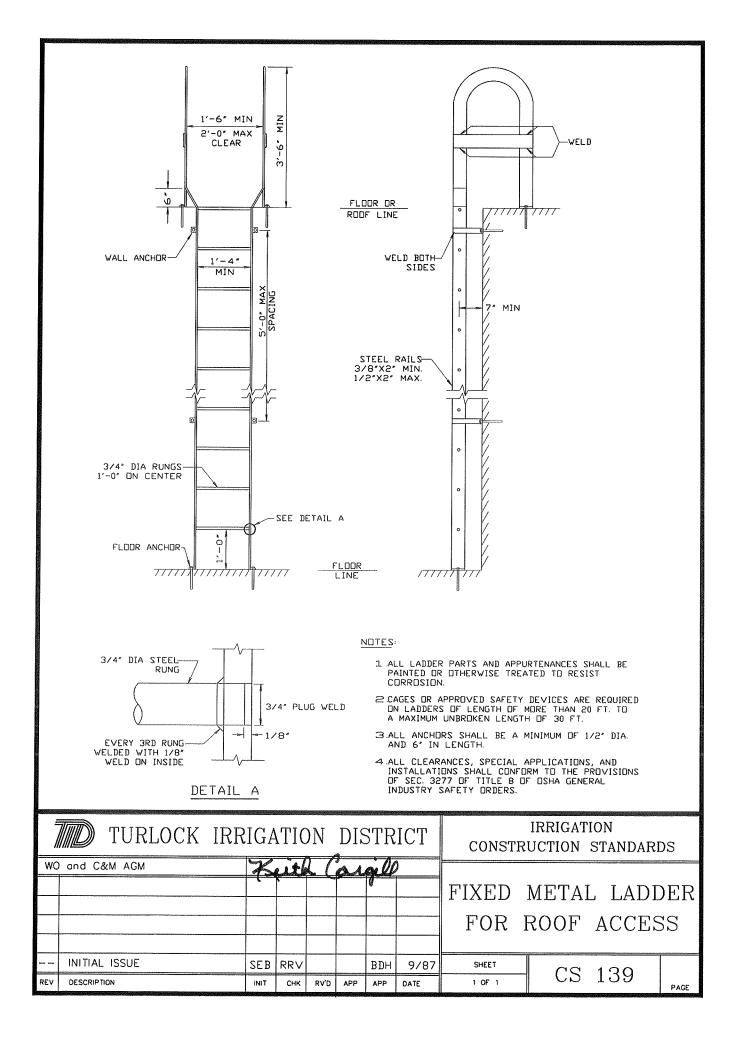
- 1) PANEL TO BE USED WITH SIERRA CONTROLS 24" X 20" NEMA ENCLOSURE.
- 2) 1/8" STRIPS TO BE STITCH WELDED TO PLATE 1/2" ON 6" PATTERN.
 (4)3/8" BOLTS TO BE INSTALLED WITH NUTS.
 - NUTS TO BE TACK WELDED IN PLACE TO BACK PLATE.
- 3) COMPLETE ASSEMBLY TO BE PAINTED OR GALVANIZED AS DETERMINED BY THE TURLOCK IRRIGATION DISTRICT.

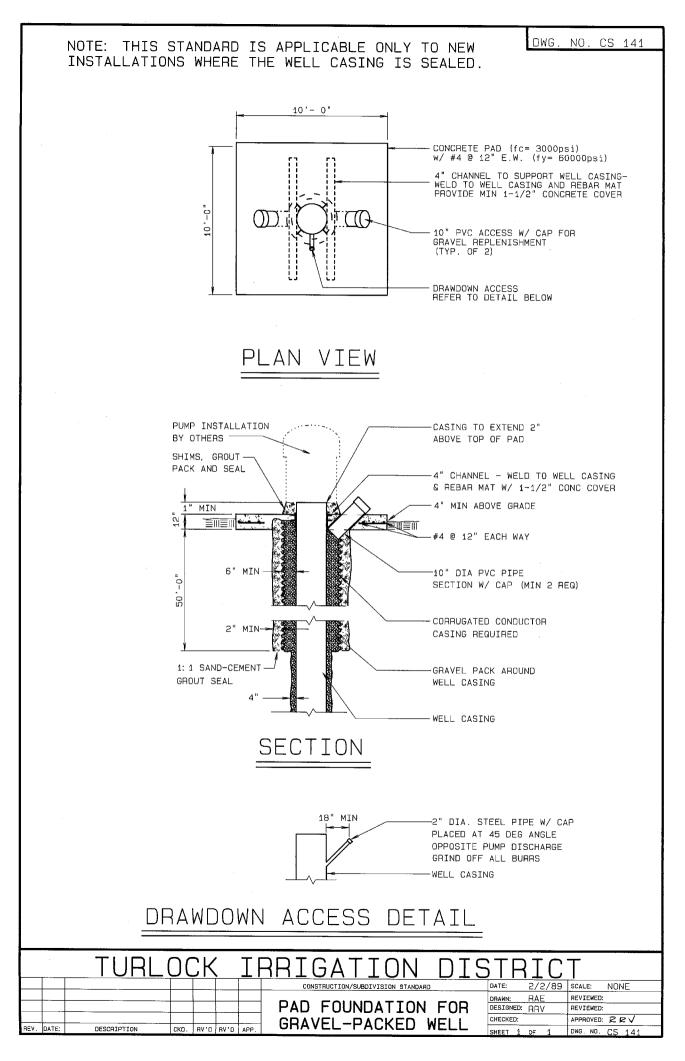


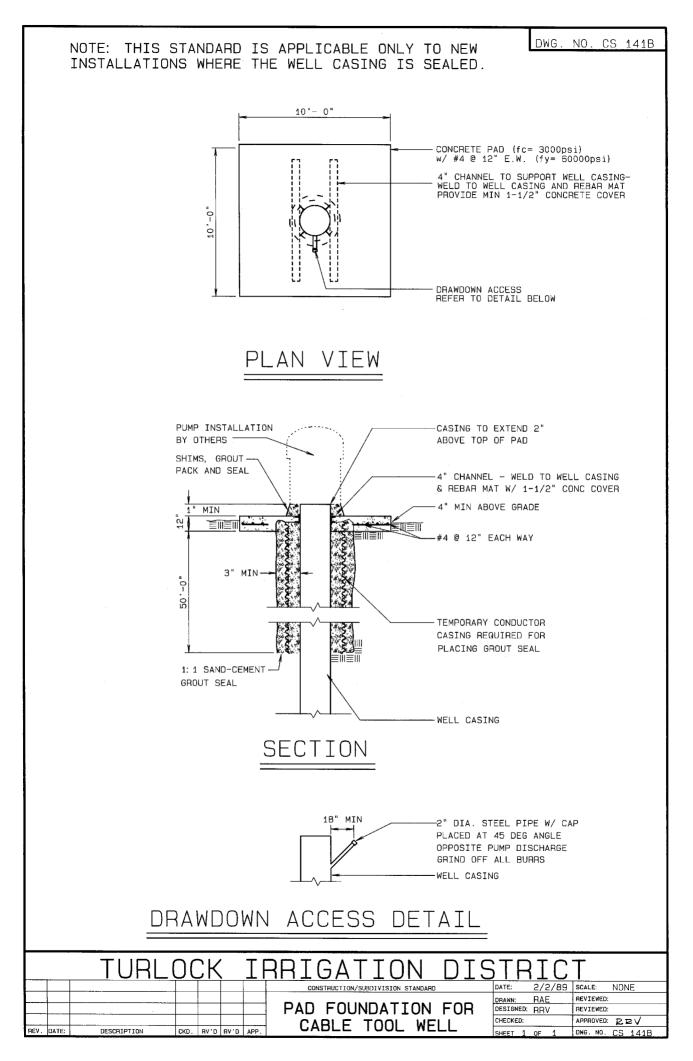




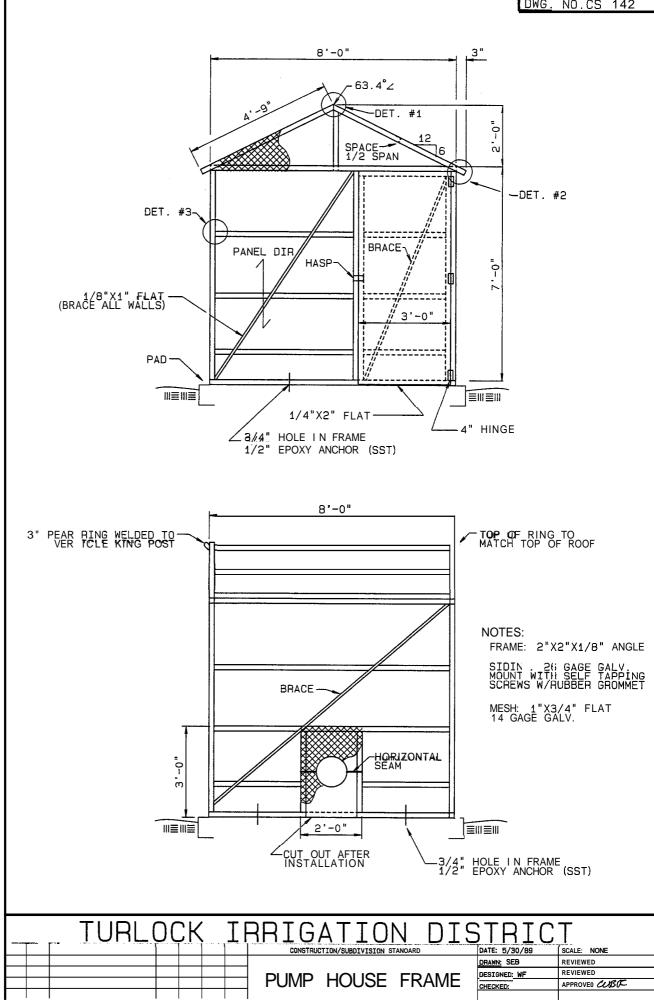


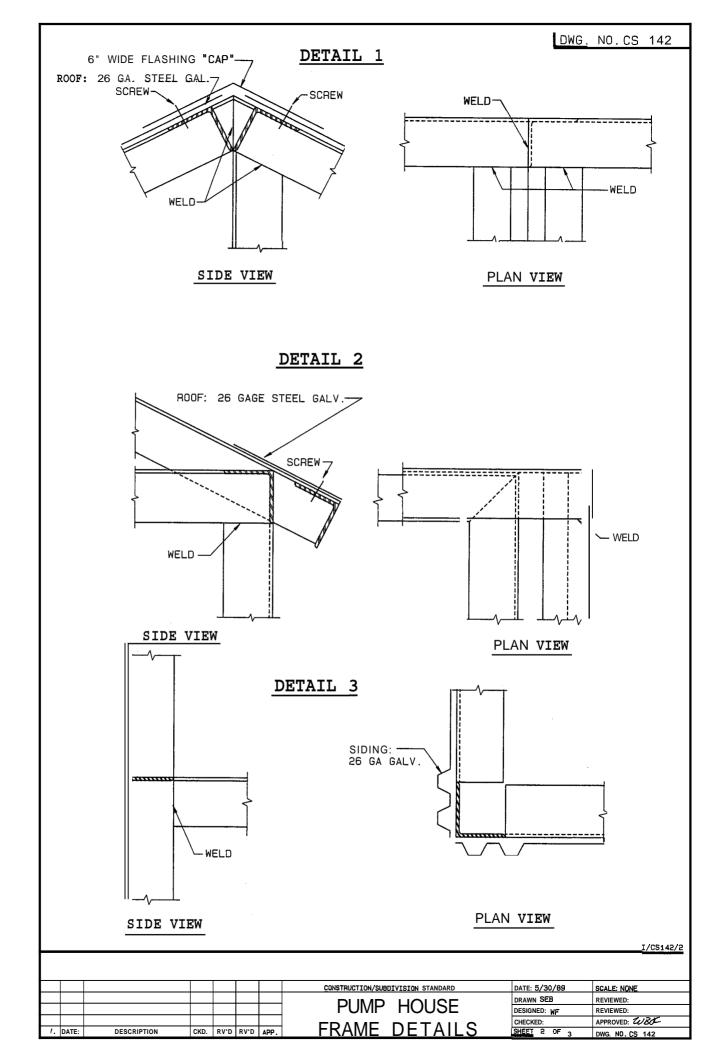


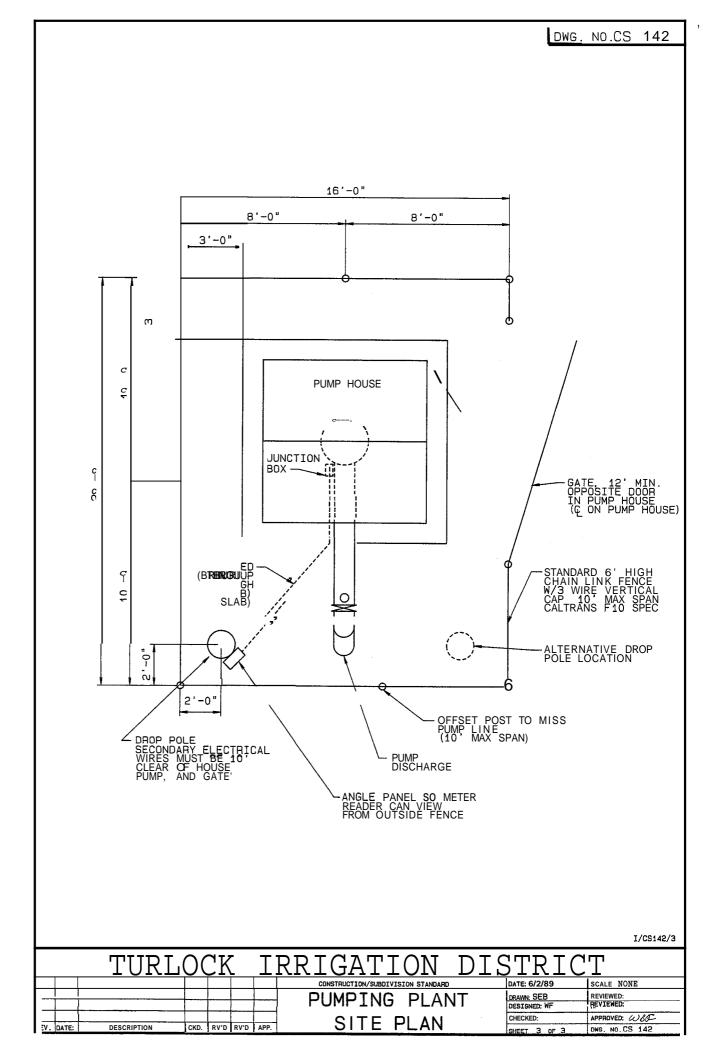


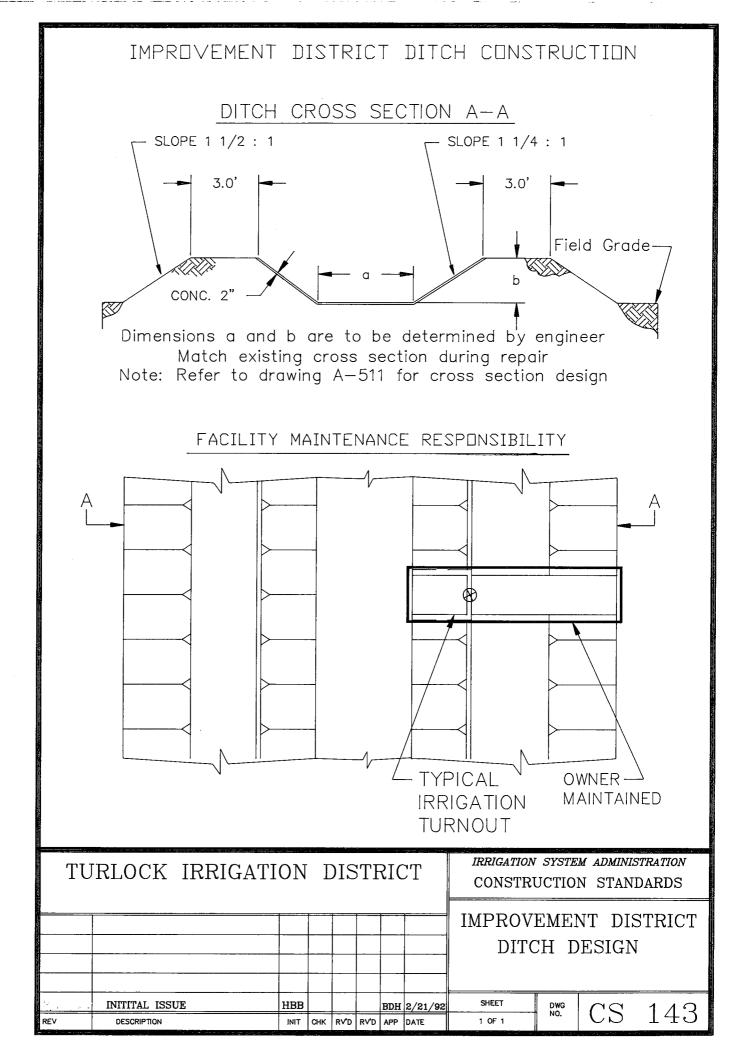


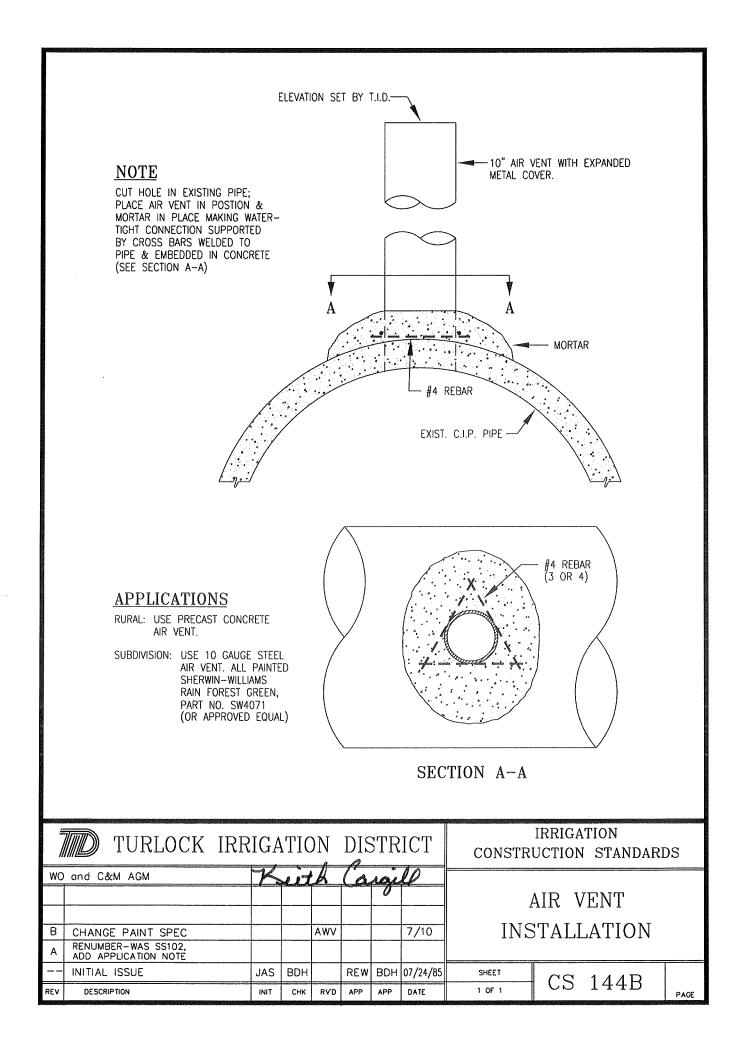
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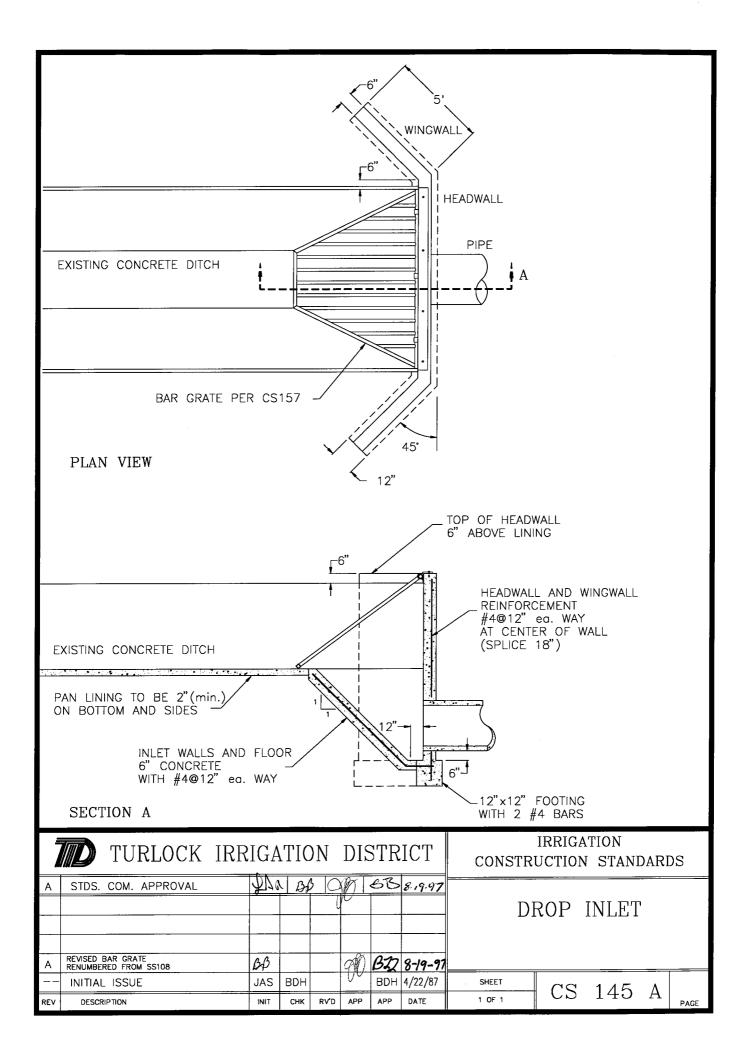


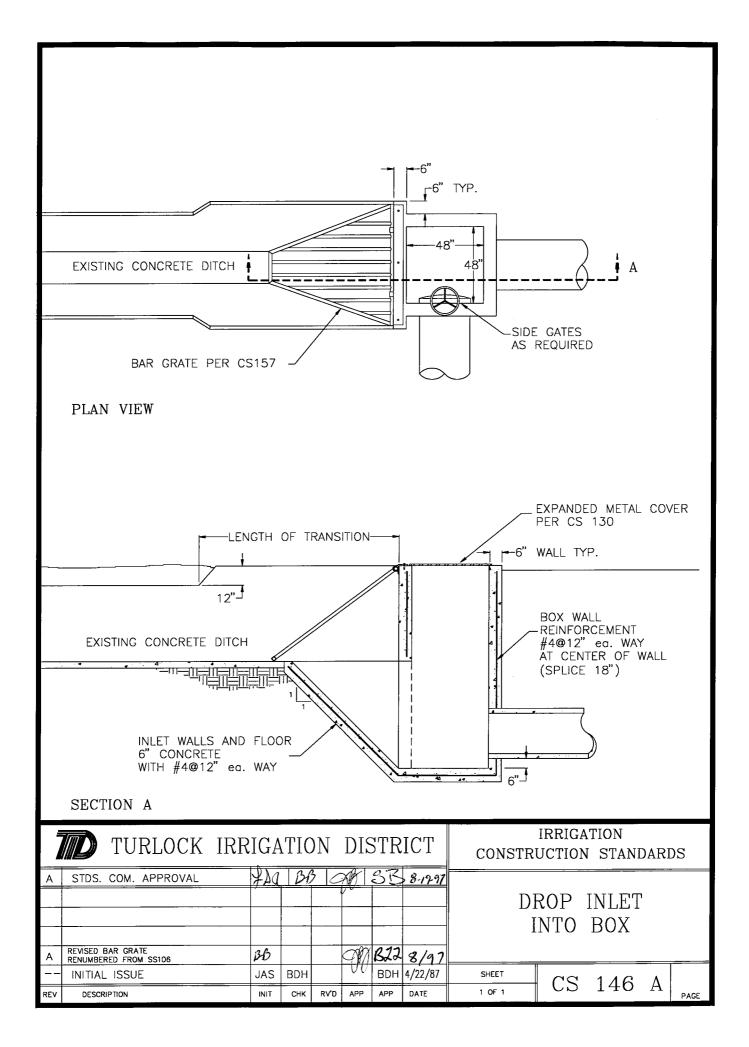


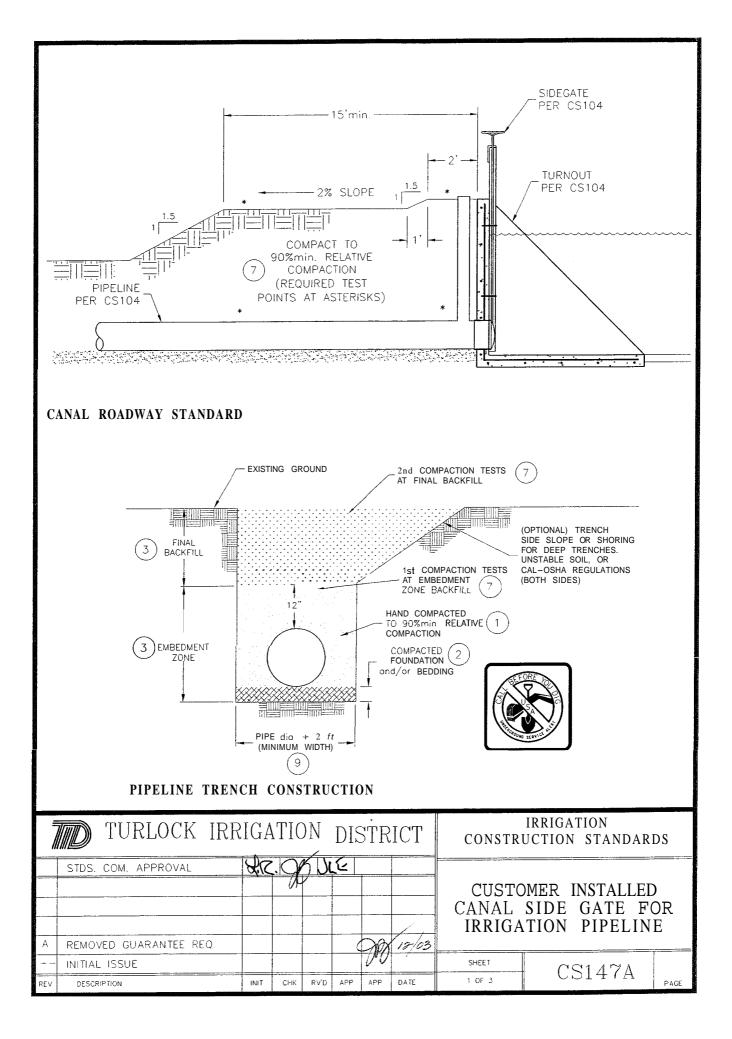












CONSTRUCTION NOTES:

- COMPACTED EARTHFILL IN THE EMBEDMENT ZONE 1. SHALL MEET 90% MINIMUM DENSITY (ASTM D-1557) AND RE ACCOMPLISHED BY MANUALLY DIRECTED COMPACTORS TO A MINIMUM DEPTH OF 12 INCHES OVER TOP OF PIPE THE MAXIMUM LAYER THICKNESS FOR EACH LIFT SHALL BE 8 INCHES BEFORE COMPACTION. ALL FINAL BACKFILL TO ALSO MEET 90% MINIMUM DENSITY. HEAVY COMPACTION EQUIPMENT SHALL NOT BE OPERATED WITHIN 2 FEET OF ANY STRUCTURE
- 2.) ALL STRUCTURE SUBGRADES ARE TO BE INSPECTED BY THE TID ENGINEERING DEPARTMENT REPRESENTATIVE PRIOR TO CONCRETE POUR OR **BACK FILL**
- 3.) EARTHFILL SHALL BE SELECT NATIVE MATERIAL, CONTAINING NO MATERIALS OVER 3 INCHES IN DIAMETER OR LENGTHS, AND BE COMPACTED AGAINST UNDISTURBED EARTH FILL MATERIALS SHALL CONTAIN NO SOD, BRUSH, ROOTS, OR OTHER ORGANIC OR UNSUITABLE MATERIAL AS DETERMINED BY THE TID ENGINEERING DEPARTMENT REPRESENTATIVE
- 4 PIPELINE SHALL BE INSTALLED ACCORDING TO MANUFACTURERS INSTRUCTIONS AND MINIMUM DEPTH OF COVER SPECIFICATIONS WITHIN THE TID RIGHT-OF-WAY SHALL BE 36 INCHES UNDER CANAL ROAD BANKS AND 18 INCHES ELSEWHERE
- 5 PIPE MATERIAL WITHIN THE TID RIGHT-OF-WAY SHALL BE ACCORDING TO THE CURRENT EDITION OF TID CONSTRUCTION STANDARD CS104
- 6 THE EDGE OF THE TID RIGHT-OF-WAY AND THE ELEVATION OF THE PIPE INVERT SHALL BE SET BY THE TID SURVEY DEPARTMENT PRIOR TO THE START OF CONSTRUCTION
- (7.) A SOIL CURVE AND FOUR COMPACTION TESTS ARE REQUIRED AND SHALL BE AT APPLICANT'S EXPENSE A SOIL CURVE MUST BE OBTAINED PRIOR TO CONSTRUCTION AND TWO COMPACTION TESTS SHALL BE DONE AT EACH STAGE OF BACKFILLING--STAGE 1 EMBEDMENT ZONE, STAGE 2 FINAL BACKFILLING (SEE SHEET 1)

- 8. DEWATERING DUE TO HIGH GROUNDWATER MAY BE REQUIRED. SHOULD GROUNDWATER BE ENCOUNTERED, THE APPLICANT SHALL CONTACT THE TID ENGINEERING DEPARTMENT REPRESENTATIVE FOR APPROVAL OF DEWATERING METHODS AND BEFORE COMMENCING BACKFILL OPERATIONS.
- (9.)TRENCH WIDTH SHALL BE 2 FEET WIDER THAN THE PIPELINE DIAMETER
- 10 LINING AT THE TURNOUT LOCATION IS TO BE SAWCUT AND REMOVED. THE REMAINING JOINT BETWEEN THE LINING AND TURNOUT IS TO BE BONDED WITH 4 INCH THICK CONCRETE KEYED UNDER THE EXISTING LINING FOR 6". AN EPOXY BONDING AGENT ("CCI CONSTRUCTION CHEMICALS LPL EPOXY BONDER" OR EQUIVALENT) IS TO BE USED AT THE JOINTS.
- 11 TURNOUT SHALL BE 6 INCH THICK POURED IN PLACE REINFORCED* CONCRETE WITH TYPE II PORTLAND CEMENT, A MAXIMUM AGGREGATE SIZE OF 3/4 INCH, AND MAXIMUM SLUMP OF 4 INCHES. CONCRETE 28 DAY COMPRESSIVE STRENGTH TO BE A MINIMUM OF 3000psi. READY MIX CONCRETE TO COMPLY WITH ASTM C-94. CONCRETE SHALL BE PREVENTED FROM DRYING FOR A CURING PERIOD OF AT LEAST 7 DAYS AFTER IT IS PLACED. EXPOSED SURFACES SHALL BE KEPT CONTINUOUSLY MOIST FOR THE ENTIRE PERIOD OR A CURING COMPOUND ("MEADOWS 1100 CURING COMPOUND" OR EQUIVALENT) MAY BE APPLIED. *TURNOUT REINFORCEMENT SHALL BE #4 BARS AT 12 INCHES EACH WAY. SEE CONSTRUCTION STANDARD CS104 FOR FURTHER DETAILS.
- 12 A CURRENT EDITION OF CONSTRUCTION STANDARD CS104 SHOULD ACCOMPANY THIS STANDARD.

TURLOCK IRRIGATION DISTRICT

IRRIGATION CONSTRUCTION STANDARDS

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CANAL IRRIGA	··•	GATE PIPEL	
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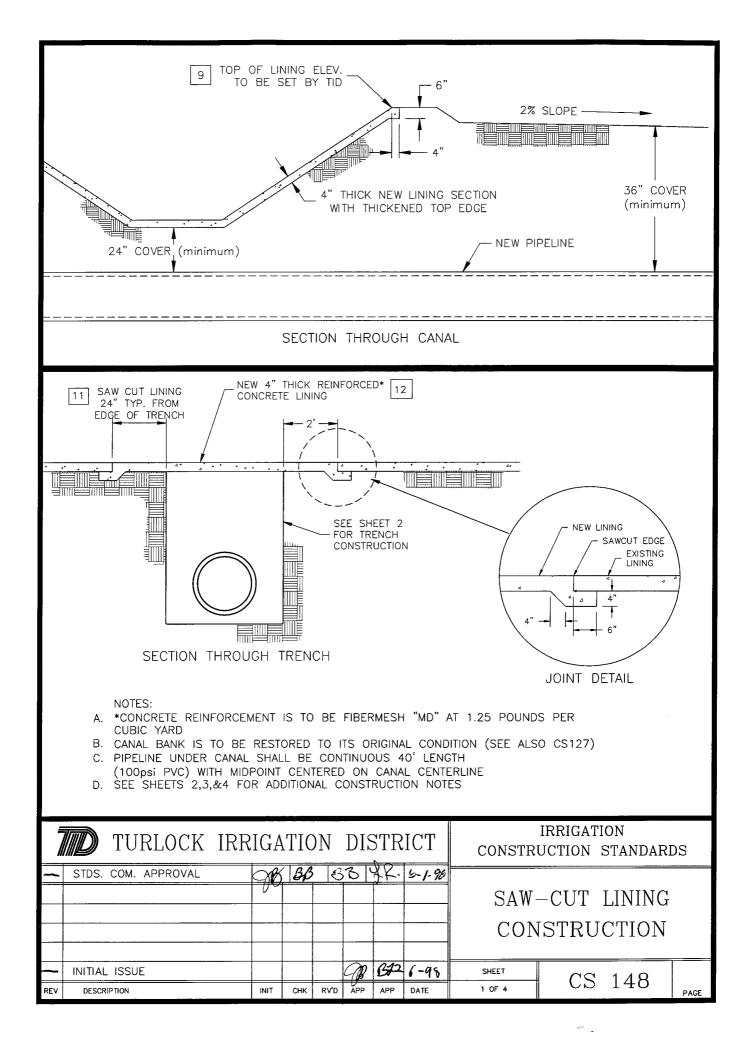
TURLOCK IRRIGATION DISTRICT NOTES:

- THESE DRAWINGS CONSTITUTE TID'S APPROVAL AS TO THE ENGINEERING ASPECTS ONLY AND DOES NOT AUTHORIZE THE CONSTRUCTION OF ANY ASPECT HEREOF OR THE INTERFERENCE WITH ANY PROPERTY, EQUIPMENT, OR INTEREST OF THE TID. NO CONSTRUCTION ON, OR INTERFERENCE WITH, TID PROPERTY SHALL OCCUR UNTIL THE TID HAS APPROVED AN "APPLICATION FOR SIDEGATE".
- 2. ALL CONSTRUCTION WORKS COMPLETED BY CUSTOMERS SHALL BE PERFORMED BY A LICENSED CONTRACTOR (STATE OF CALIFORNIA) APPROVED BY THE TID ENGINEERING DEPARTMENT REPRESENTATIVE PRIOR TO COMMENCING WORK.
- 3. ALL SIDEGATES INSTALLED BY CUSTOMERS WILL REQUIRE A \$4,000 PERFORMANCE BOND OR EQUIVALENT CASH DEPOSIT PRIOR TO COMMENCING WORK.
- 4. ALL CONSTRUCTION WITHIN THE TID RIGHT-OF-WAY SHALL BE DONE IN ACCORDANCE WITH THE ATTACHED DRAWINGS AND THE CURRENT EDITION OF TID CONSTRUCTION STANDARD(s) CS104.
- 5. CONSTRUCTION WITHIN THE TID RIGHT-OF-WAY WILL NOT BE ALLOWED DURING THE IRRIGATION SEASON (GENERALLY MARCH 1 TO NOVEMBER 1).
- 6. WHERE THE PLANS OR SPECIFICATIONS DESCRIBE PORTIONS OF THE WORK IN GENERAL TERMS, BUT NOT IN COMPLETE DETAIL, IT IS UNDERSTOOD THAT ONLY THE BEST GENERAL PRACTICE IS TO PREVAIL AND THAT ONLY MATERIALS AND WORKMANSHIP OF THE FIRST QUALITY ARE TO BE USED.
- 7. APPLICANT AGREES TO ASSUME SOLE AND COMPLETE RESPONSIBILITY FOR JOB SITE CONDITIONS DURING THE COURSE OF CONSTRUCTION OF THIS PROJECT INCLUDING SAFETY OF ALL PERSONS AND PROPERTY. THIS REQUIREMENT SHALL APPLY CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS.
- 8. CAL-OSHA SAFETY REQUIREMENTS SHALL BE IN EFFECT DURING ALL CONSTRUCTION. SPECIAL SAFETY PRECAUTIONS SHALL BE TAKEN WHEN WORKING IN THE VICINITY OF GAS, OIL, OR ELECTRICAL LINES.
- 9. APPLICANT SHALL OBTAIN A DIG ALERT IDENTIFICATION NUMBER AT LEAST TWO WORKING DAYS BEFORE DIGGING UNDERGROUND.
- 10. THE TID WILL NOT BE RESPONSIBLE OR LIABLE FOR UNAUTHORIZED CHANGES TO, OR USES OF THESE PLANS. ALL CHANGES TO THE PLANS MUST BE OBTAINED IN WRITING FROM THE TID ENGINEERING DEPARTMENT REPRESENTATIVE.

- 11. ALL IMPROVEMENTS SHALL BE PAID FOR BY THE APPLICANT THE TID WILL BE THE OWNER OF AND WILL MAINTAIN ALL IMPROVEMENTS WITHIN THE TID RIGHT-OF-WAY
- 12. ANY DAMAGES TO TID FACILITIES DURING CONSTRUCTION SHALL BE REPAIRED OR REPLACED PER THE TID CONSTRUCTION STANDARDS AT THE SOLE COST OF THE APPLICANT.
- 13. SCHEDULE FOR SIDEGATE INSTALLATION SHALL BE APPROVED BY THE TID WATER DISTRIBUTION DEPARTMENT MANAGER PRIOR TO COMMENCING WORK.
- 14. INSPECTIONS CONCERNING VARIOUS STAGES OF WORK WILL BE REQUIRED, AS STATED IN THE ATTACHED SPECIFICATIONS, AT THE APPLICANT'S EXPENSE. THEY INCLUDE: A. SOIL COMPACTION TESTING
 - B. INSPECTION OF FOUNDATIONS, FORMS, AND REINFORCING STEEL PRIOR TO PLACING CONCRETE
 - C. POST CONSTRUCTION ACCEPTANCE.
- 15. CONTACT THE TURLOCK IRRIGATION DISTRICT ENGINEERING DEPARTMENT AT LEAST FIVE (5) WORKING DAYS PRIOR TO COMMENCING CONSTRUCTION, AT LEAST ONE (1) WORKING DAY PRIOR TO POURING CONCRETE, AND WITHIN FIVE (5) WORKING DAYS AFTER SUCH WORK IS COMPLETED. WORK WITHIN THE TID RIGHT-OF-WAY SHALL PROCEED IN A CONTINUOUS MANNER ONCE STARTED. THE TID ENGINEERING DEPARTMENT SHALL BE NOTIFIED OF ANY WORK STOPPAGES. WHENEVER WORK IS TO RESTART, THE TID ENGINEERING DEPARTMENT SHALL REQUIRE AN ADDITIONAL TWO (2) WORKING DAY NOTICE. THE TID ENGINEERING DEPARTMENT SHALL ALSO BE CONTACTED A MINIMUM OF TWO (2) WORKING DAYS PRIOR TO ALL CONSTRUCTION SCHEDULED ON A HOLIDAY OR WEEKEND.
- 16. APPLICANT MUST NOTIFY TID UPON COMPLETION OF CONSTRUCTION IN ORDER FOR THE TID ENGINEER TO MAKE A FINAL INSPECTION FOR ACCEPTANCE OF THE IMPROVEMENTS.

APPROVED BY TURLOCK IRRIGATION DISTRICT FOR IRRIGATION FACILITIES ONLY.

THE TURLOCK IRRIGATION DISTRICT	CUSTOMER INSTALLED CANAL SIDE GATE FOR					
IRRIGATION	IRRIGATION PIPELINE					
CONSTRUCTION STANDARDS	SHEET	CS147A				
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EXISTING GROUND (OR BOTTOM OF LINING)	
4 FINAL 8 3' MIN. COVER TYP. (2' MIN. UNDER BOTTOM OF LINING) 10 BEDDING 2 FOUNDATION	(OPTIONAL) TRENCH SIDE SLOPE OR SHORING FOR DEEP TRENCHES, UNSTABLE SOIL, OR CAL-OSHA REGULATIONS (BOTH SIDES) HAND COMPACTED TO 90% RELATIVE COMPACTION (MIN.) (ASTM D-1557) EMBEDMENT 1 ZONE 1
SECTION THROUGH TRE	ENCH
NOTES: A. ALL COMPACTION TO BE DONE TO 90% RELATIVE COMPACTION B. FINAL BACKFILL TO BE SUITABLE C. SEE SHEETS 3&4 FOR ADDITION	(MINIMUM) (ASTM D-1557) E NATIVE MATERIAL
TID TURLOCK IRRIGATION DISTRICT	SAW-CUT LINING CONSTRUCTION
IRRIGATION CONSTRUCTION STANDARDS	SHEET CS 148 PAGE

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CONSTRUCTION NOTES:

- 1. COMPACTED EARTHFILL IN THE EMBEDMENT ZONE SHALL MEET 90% MINIMUM DENSITY (ASTM D-1557) AND BE ACCOMPLISHED BY MANUALLY DIRECTED COMPACTORS TO A MINIMUM DEPTH OF 12 INCHES OVER TOP OF PIPE. THE MAXIMUM LAYER THICKNESS SHALL BE 8 INCHES BEFORE COMPACTION. (SEE SHEET 2) HEAVY COMPACTION EQUIPMENT SHALL NOT BE OPERATED WITHIN 2 FEET OF ANY STRUCTURE.
- 2. ALL STRUCTURE SUBGRADES ARE TO BE INSPECTED BY THE TID ENGINEERING DEPARTMENT REPRESENTATIVE PRIOR TO CONCRETE POUR OR BACKFILL AND SHALL MEET 90% MINIMUM DENSITY.
- 3. EARTHFILL SHALL BE SELECT NATIVE MATERIAL CONTAINING NO MATERIALS OVER 3 INCHES IN DIAMETER OR LENGTH, AND BE COMPACTED AGAINST UNDISTURBED EARTH. FILL MATERIALS SHALL CONTAIN NO SOD, BRUSH, ROOTS, OR OTHER ORGANIC OR UNSUITABLE MATERIAL AS DETERMINED BY THE TID ENGINEERING DEPARTMENT REPRESENTATIVE.
- 4. THE PIPELINE SHALL BE INSTALLED ACCORDING TO MANUFACTURERS INSTRUCTIONS AND SPECIFICATIONS. MINIMUM DEPTH OF COVER SHALL BE 36 INCHES WITHIN TID CANAL BANKS AND RIGHT-OF-WAYS EXCEPT UNDER CHANNEL BOTTOMS WHERE THE MINIMUM COVER SHALL BE 24 INCHES. PIPELINE CASING, IF USED, SHALL CONFORM TO TID STANDARD ES110.
- 5. TID REQUIRED INSPECTIONS AND COMPACTION TESTS SHALL BE AT APPLICANT'S SOLE EXPENSE. ANY RETEST SHALL BE PAID BY THE APPLICANT.
- 6. DEWATERING DUE TO HIGH GROUNDWATER MAY BE REQUIRED. SHOULD GROUNDWATER BE ENCOUNTERED, THE APPLICANT SHALL CONTACT THE TID ENGINEERING DEPARTMENT REPRESENTATIVE FOR APPROVAL OF DEWATERING METHODS AND BEFORE COMMENCING BACKFILL OPERATIONS.
- 7. TRENCH WIDTHS SHALL BE AS SHOWN UNLESS THE PIPELINE SIZE IS 4 INCHES OR SMALLER-WHERE THE TRENCH WIDTH SHALL HAVE A MINIMUM WIDTH OF 12 INCHES.
- 8. ALL FINAL BACKFILLING SHALL HAVE MINIMUM 90% RELATIVE COMPACTION UNLESS OTHER LOCAL AGENCY STANDARDS DICTATE OTHERWISE.
- 9. THE ELEVATION OF THE TOP OF LINING IS TO BE SET BY THE TID SURVEY DEPARTMENT PRIOR TO THE START OF CONSTRUCTION.

- 10. BEDDING SHALL BE REQUIRED FOR REINFORCED CONCRETE PIPE INSTALLATIONS. BEDDING SHALL BE MINIMUM 4 INCHES DEEP' OR AS SPECIFIED BY THE TID ENGINEERING DEPARTMENT REPRESENTATIVE. WHEN BEDDING IS REQUIRED A CLAY PLUG SHALL BE CONSTRUCTED UPSTREAM OF EACH MANHOLE OR AT 400 FOOT INTERVALS. THE CLAY PLUG SHALL HAVE A MINIMUM 12 INCH WIDTH AND EXTEND A MINIMUM OF 12 INCHES INTO UNDISTURBED TRENCH WALLS, TRENCH BOTTOM, AND ABOVE TOP OF PIPE. BEDDING SHALL MEET 90% MINIMUM DENSITY AND CONFORM TO THE SPECIFICATIONS BELOW. SOIL TYPES SHALL BE AS DETERMINED BY THE TID ENGINEERING DEPARTMENT REPRESENTATIVE. OTHER BEDDING MATERIALS MAY BE USED WITH PRIOR APPROVAL BY THE TID ENGINEERING DEPARTMENT REPRESENTATIVE. Α
 - <u>ON SANDY SOIL</u> BEDDING:
 - 1 1/2 INCH MAX. AGGREGATE-CLASS 2 B. ON CLAY SOIL
 - BEDDING: 1 1/2 INCH MAX. AGGREGATE-CLASS 2 HAUNCHING:
 - SAND SHALL BE PLACED IN MAX. 8 INCH LIFTS IN HAUNCHING ZONE
- 11. APPLICANT SHALL INSTALL NEW CONCRETE LINING AS PER TID STANDARDS AND ATTACHED DETAILS. A MINIMUM OF 24 INCHES ON EACH SIDE OF THE TRENCH AREA (OR AS DIRECTED BY THE TID ENGINEERING DEPARTMENT REPRESENTATIVE) SHALL BE SAWCUT AND REMOVED. SOIL IN THE TRENCH AREA SHALL BE COMPACTED TO 90% AND MOISTENED PRIOR TO PLACEMENT OF NEW CONCRETE.
- 12. CONCRETE LINING SHALL BE POURED IN PLACE REINFORCED* CONCRETE WITH TYPE II PORTLAND CEMENT, A MAXIMUM AGGREGATE SIZE OF 1/2 INCH, AND MAXIMUM SLUMP OF 4 INCHES. CONCRETE 28 DAY COMPRESSIVE STRENGTH TO BE A MINIMUM OF 3000psi. CONCRETE SHALL BE PREVENTED FROM DRYING FOR A CURING PERIOD OF AT LEAST 7 DAYS AFTER IT IS PLACED. EXPOSED SURFACES SHALL BE KEPT CONTINUOUSLY MOIST FOR THE ENTIRE PERIOD OR A CURING COMPOUND ("MEADOWS 1100 CURING COMPOUND" OR EQUIVALENT) MAY BE APPLIED. *REINFORCEMENT SHALL BE "FIBERMESH MD" AT 1.25 POUNDS PER CUBIC YARD OF CONCRETE (OR PRE-APPROVED EQUIVALENT).
- 13. UTILITY MARKERS AND POSTS SHALL BE INSTALLED ACCORDING TO TID STANDARD ES110

TURLOCK IRRIGATION DISTRICT	SAW-CUT LINING CONSTRUCTION			
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CONSTRUCTION STANDARDS	SHEET	CS 148		
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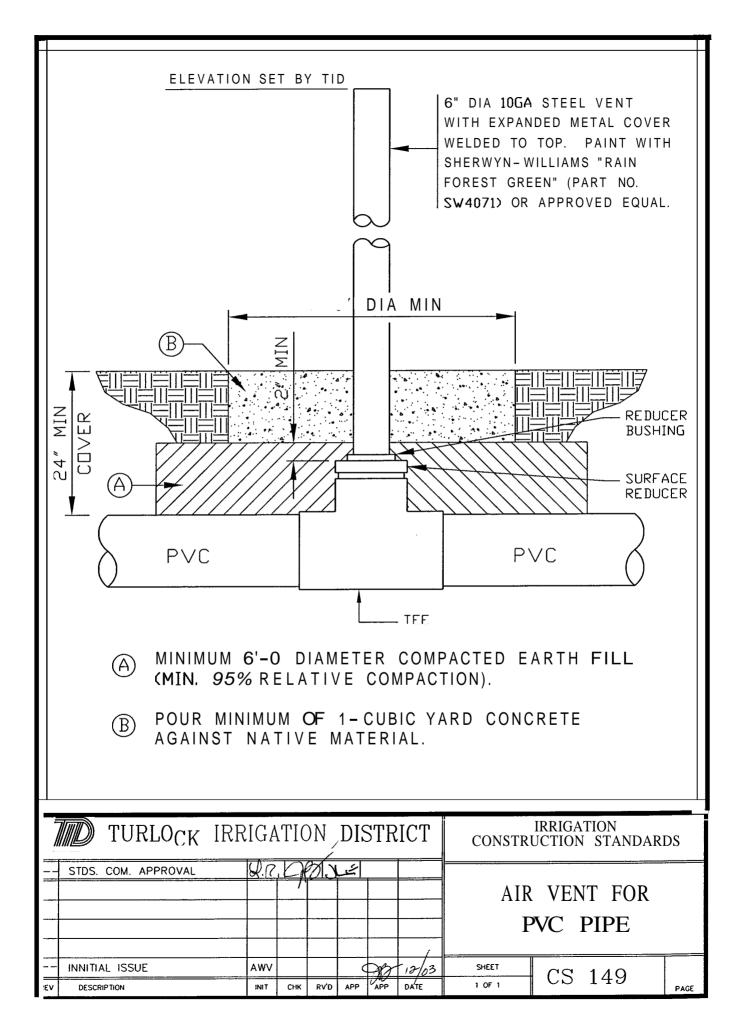
TURLOCK IRRIGATION DISTRICT (TID) NOTES:

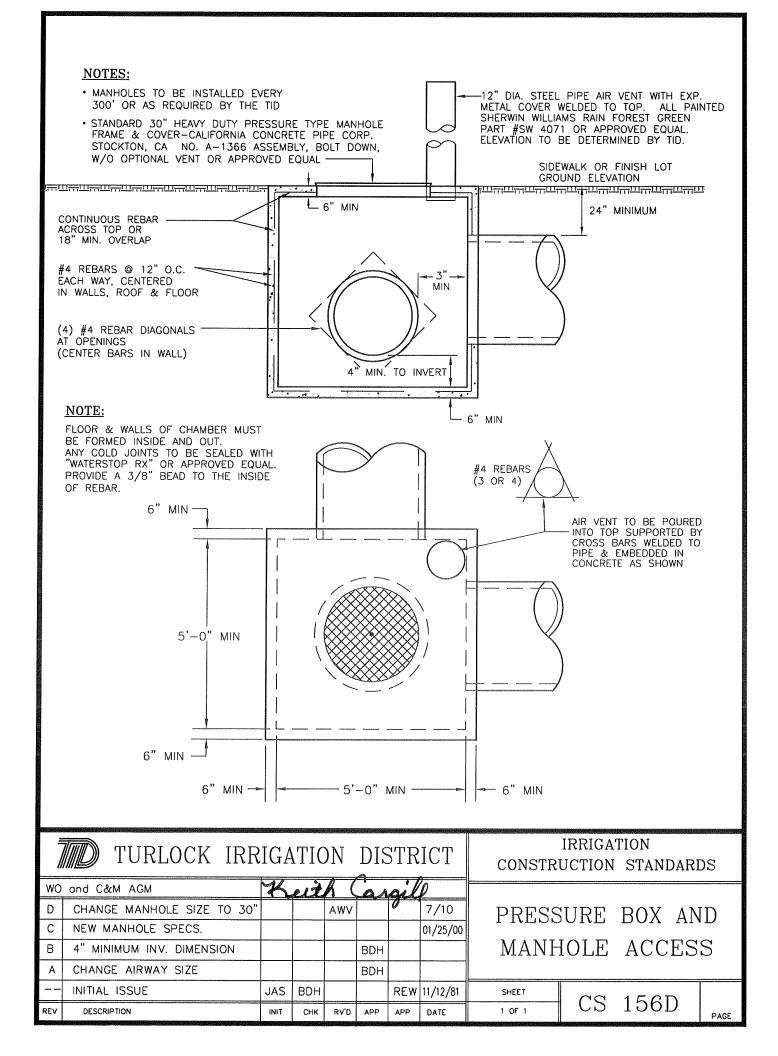
- THESE DRAWINGS CONSTITUTE TID'S APPROVAL AS TO THE ENGINEERING ASPECTS ONLY AND DOES NOT AUTHORIZE THE CONSTRUCTION OF ANY ASPECT HEREOF OR THE INTERFERENCE WITH ANY PROPERTY, EQUIPMENT, OR INTERFERENCE WITH, NO CONSTRUCTION ON, OR INTERFERENCE WITH, TID PROPERTY SHALL OCCUR UNTIL THE TID HAS APPROVED AN "APPLICATION FOR PIPELINE INSTALLATION".
- 2. ALL CONSTRUCTION WORKS COMPLETED BY APPLICANT SHALL BE PERFORMED BY A LICENSED CONTRACTOR (STATE OF CALIFORNIA) TO BE APPROVED BY THE TID ENGINEERING DEPARTMENT REPRESENTATIVE PRIOR TO COMMENCING WORK. THE APPLICANT'S CONTRACTOR SHALL BE REQUIRED TO POST A PERFORMANCE BOND.
- 3. ALL CONSTRUCTION WITHIN THE TID RIGHT-OF-WAY SHALL BE DONE IN ACCORDANCE WITH THE ATTACHED DRAWINGS AND THE CURRENT EDITION OF TID CONSTRUCTION STANDARDS.
- 4. CONSTRUCTION WITHIN THE TID RIGHT-OF-WAY WILL NOT BE ALLOWED DURING THE IRRIGATION SEASON (GENERALLY MARCH 1 TO NOVEMBER 1).
- 5. A PROJECT SCHEDULE SHALL BE APPROVED BY THE TID WATER DISTRIBUTION DEPARTMENT MANAGER PRIOR TO COMMENCING WORK.
- 6. WHERE THE PLANS OR SPECIFICATIONS DESCRIBE PORTIONS OF THE WORK IN GENERAL TERMS, BUT NOT IN COMPLETE DETAIL, IT IS UNDERSTOOD THAT ONLY THE BEST GENERAL PRACTICE IS TO PREVAIL AND THAT ONLY MATERIALS AND WORKMANSHIP OF THE FIRST QUALITY ARE TO BE USED.
- 7. APPLICANT AGREES TO ASSUME SOLE AND COMPLETE RESPONSIBILITY FOR JOB SITE CONDITIONS DURING THE COURSE OF CONSTRUCTION OF THIS PROJECT INCLUDING SAFETY OF ALL PERSONS AND PROPERTY. THIS REQUIREMENT SHALL APPLY CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS.
- 8: CAL-OSHA SAFETY REQUIREMENTS SHALL BE IN EFFECT DURING ALL CONSTRUCTION. SPECIAL SAFETY PRECAUTIONS SHALL BE TAKEN WHEN WORKING IN THE VICINITY OF GAS, OIL, OR ELECTRICAL LINES.
- 9. APPLICANT SHALL OBTAIN A DIG ALERT IDENTIFICATION NUMBER AT LEAST TWO WORKING DAYS BEFORE DIGGING UNDERGROUND.
- 10. ANY DAMAGES TO TID FACILITIES DURING CONSTRUCTION SHALL BE REPAIRED OR REPLACED PER THE TID CONSTRUCTION STANDARDS AT THE SOLE COST OF THE APPLICANT.

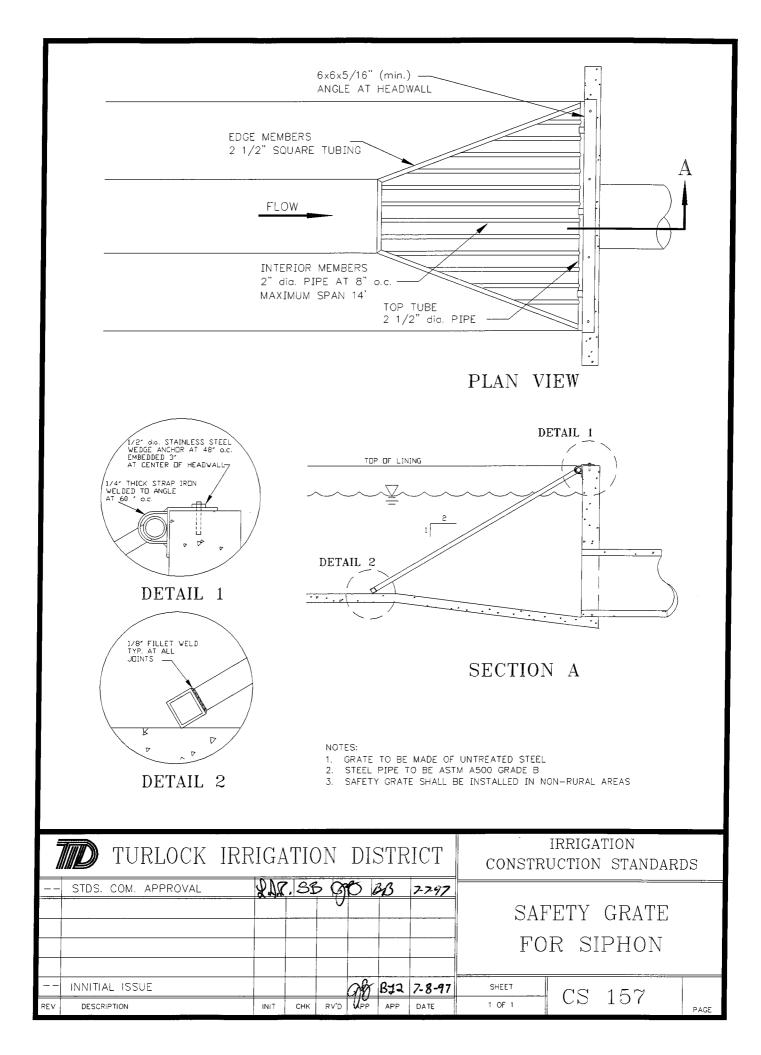
- 11. THE APPLICANT GUARANTEES ALL CONSTRUCTED IRRIGATION WORKS AND IMPROVEMENTS WITHIN THE TID RIGHT-OF-WAY FOR A PERIOD OF ONE YEAR FOLLOWING COMPLETION OF CONSTRUCTION AND ACCEPTANCE BY TID. THE GUARANTEE INCLUDES REPAIR BY THE APPLICANT OR PAYMENT BY THE APPLICANT FOR REPAIRS DONE BY TID. SEE THE ATTACHED "ONE YEAR GUARANTEE" FORM FOR FURTHER DETAILS.
- 12. THE TID WILL NOT BE RESPONSIBLE OR LIABLE FOR UNAUTHORIZED CHANGES TO, OR USES OF, THESE PLANS. ALL CHANGES TO THE PLANS MUST BE OBTAINED IN WRITING FROM THE TID ENGINEERING DEPARTMENT REPRESENTATIVE.
- 13. THE APPLICANT SHALL BE REQUIRED TO HAVE A PRE-CONSTRUCTION CONFERENCE WITH THE TID ENGINEERING DEPARTMENT REPRESENTATIVE PRIOR TO STARTING ANY WORK WITHIN THE TID RIGHT-OF-WAY.
- 14. INSPECTIONS CONCERNING VARIOUS STAGES OF WORK WILL BE REQUIRED, AS STATED IN THE ATTACHED SPECIFICATIONS, AT THE APPLICANT'S EXPENSE. THEY INCLUDE: A. SOIL COMPACTION TESTING B. INSPECTION, OF FOUNDATIONS, FORMS
 - B. INSPECTION OF FOUNDATIONS, FORMS, AND REINFORCING STEEL PRIOR TO PLACING CONCRETE
 - C. POST CONSTRUCTION ACCEPTANCE.
- 15. CONTACT THE TURLOCK IRRIGATION DISTRICT ENGINEERING DEPARTMENT AT LEAST FIVE (5) WORKING DAYS PRIOR TO COMMENCING CONSTRUCTION, AT LEAST ONE (1) WORKING DAY PRIOR TO POURING CONCRETE, AND WITHIN FIVE (5) WORKING DAYS AFTER SUCH WORK IS COMPLETED. WORK WITHIN THE TID RIGHT-OF-WAY SHALL PROCEED IN A CONTINUOUS MANNER ONCE STARTED. THE TID ENGINEERING DEPARTMENT SHALL BE NOTIFIED OF ANY WORK STOPPAGES. WHENEVER WORK IS TO RESTART, THE TID ENGINEERING DEPARTMENT SHALL REQUIRE AN ADDITIONAL TWO (2) WORKING DAY NOTICE. THE TID ENGINEERING DEPARTMENT SHALL ALSO BE CONTACTED A MINIMUM OF TWO (2) WORKING DAYS PRIOR TO ALL CONSTRUCTION SCHEDULED ON A HOLIDAY OR WEEKEND.
- 16. APPLICANT MUST NOTIFY TID UPON COMPLETION OF CONSTRUCTION IN ORDER FOR THE TID ENGINEERING REPRESENTATIVE TO MAKE A FINAL INSPECTION FOR ACCEPTANCE OF THE IMPROVEMENTS. ANY WORK WITHIN THE TID RIGHT-OF-WAY SHALL NOT BE DEEMED COMPLETE UNTIL THE TID ENGINEERING DEPARTMENT HAS BEEN PROVIDED WITH A SET OF AS-BUILT PLANS.

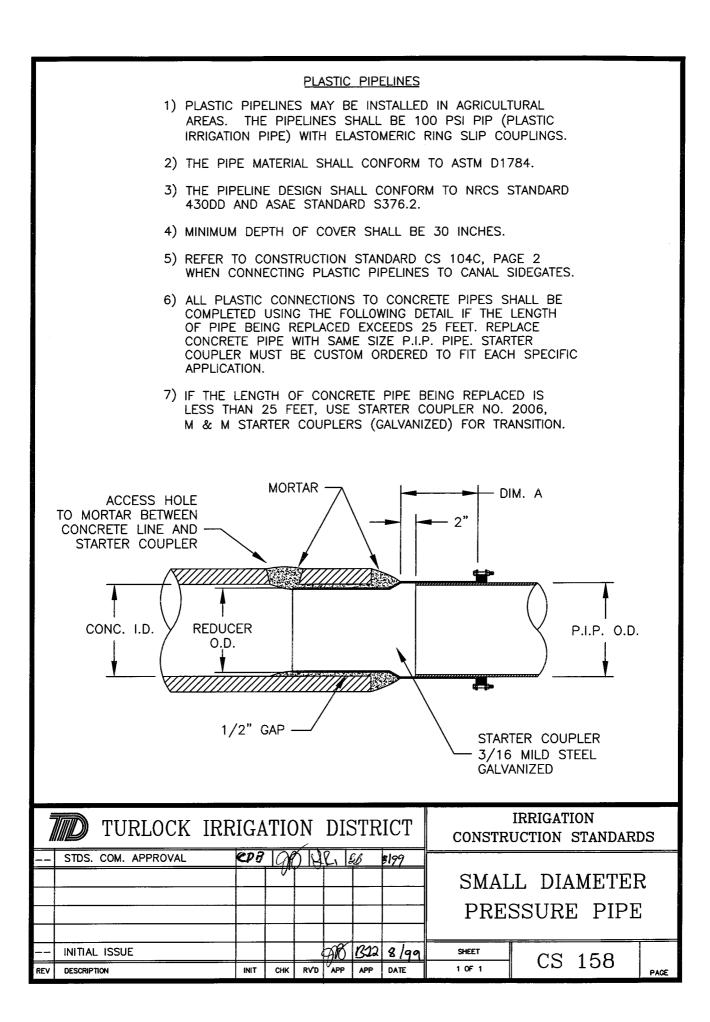
APPROVED BY TURLOCK IRRIGATION DISTRICT FOR IRRIGATION FACILITIES ONLY.

	TURLOCK IRRIGATION DISTRICT	SAW-CUT LINING CONSTRUCTION				
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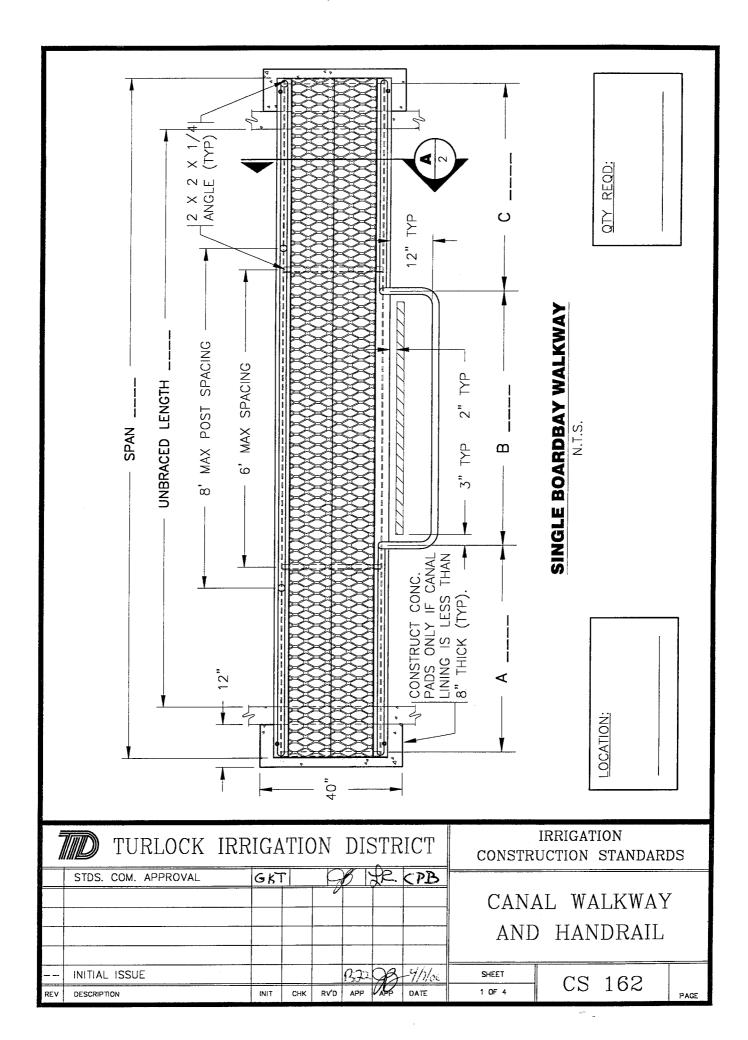


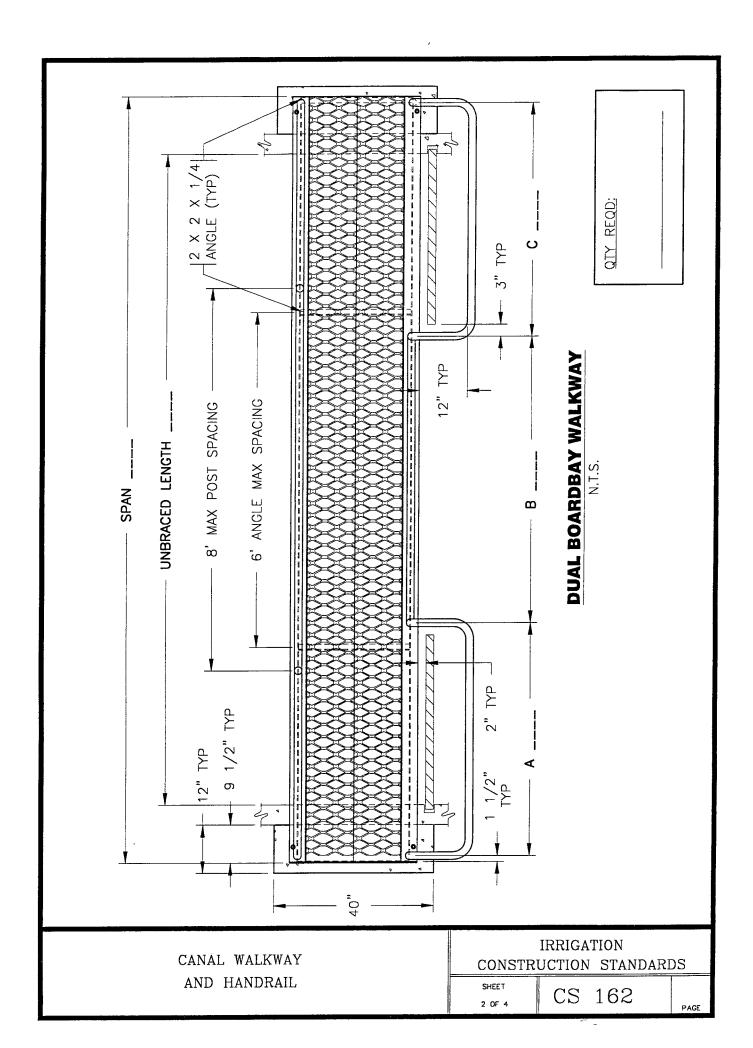


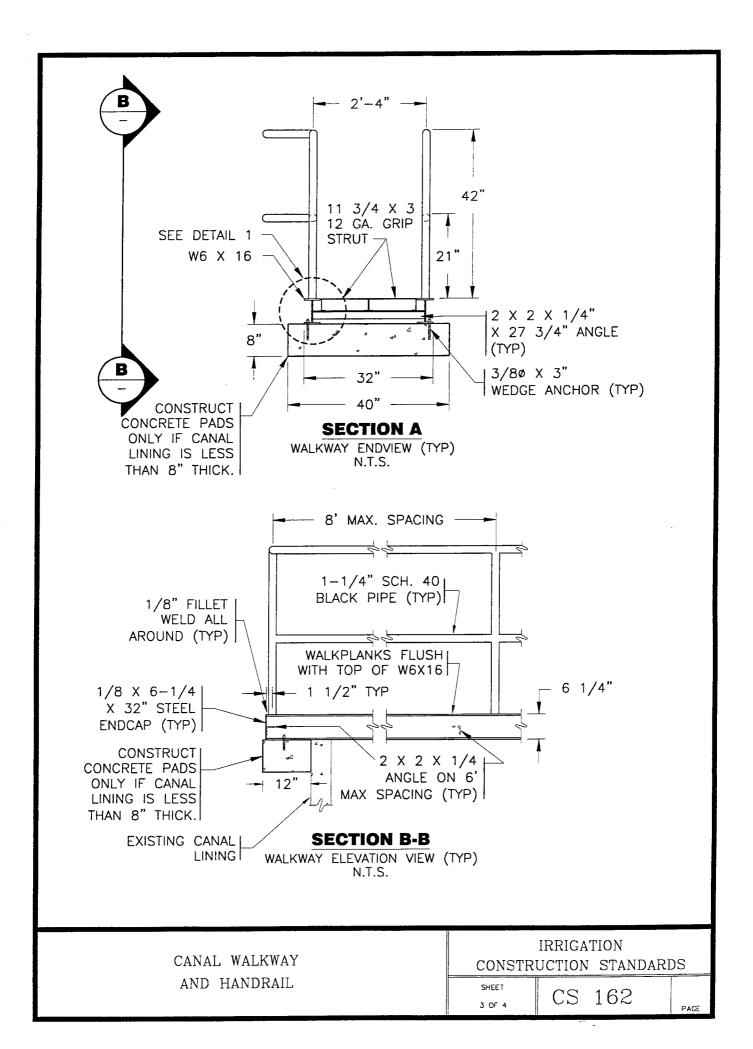


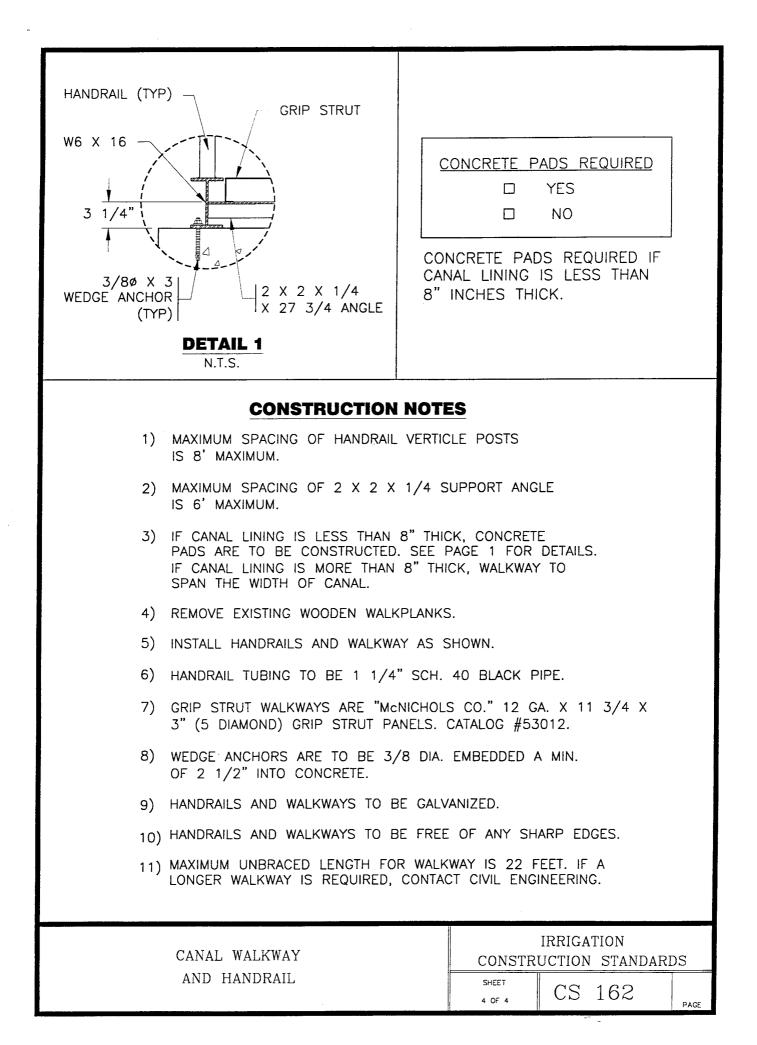


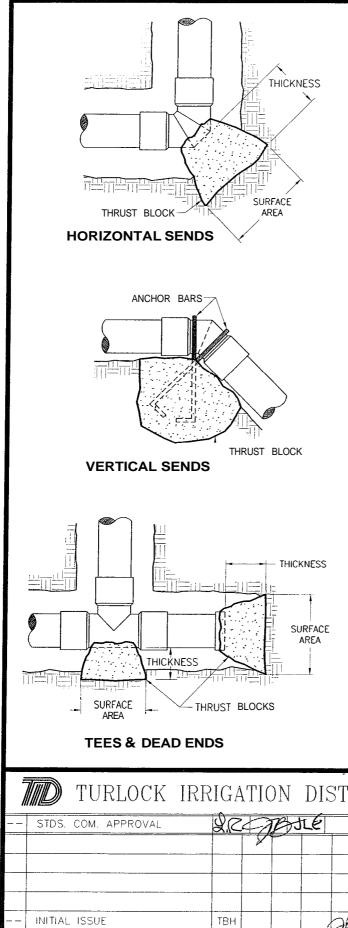
EXPANDED METAL WELD TO PLATE		3/4" BRASS PLUG
	IGATION DISTRICT	IRRIGATION CONSTRUCTION STANDARDS
STDS. COM. APPROVAL	3B CO L.R.	FLOW METER TAP FOR CONCRETE PIPE
INITIAL ISSUE REV DESCRIPTION	INIT CHK RVD УР АРР ДАТЕ	SHEET CS 159











	,				
DIAMETER	HORIZONTA	L BENDS	VERTICAL BENDS		
OF PIPE	SURFACE AREA	THICKNESS	WEIGHT		
(INCHES)	(SO. FT.)	(INCHES)	(LBS)		
22.5' BENI	1				
8	1	6	1000		
10	1	6	1600		
12	1.5	8	2200		
14	1.5	8	3000		
16	2	12	4000		
18	2.5	12	5000		
20	3	16	6200		
45' BENDS					
8	1	8	2000		
10	1.5	12	3000		
12	2.5	14	4400		
14	3	14	5900		
16	4	18	7700		
18	5	18	9800		
20	6	20	12000		
90" BENDS					
8	2	12	3600		
10	3	14	5600		
12	4	14	8000		
14	5.5	18	10900		
16	7	18	14200		
18	9	24	18000		
20	11	24	22200		
TEES & DEA	D ENDS				
8	1.5	12	-		
10	2	14	_		
12	3	14	_		
14	4	18	-		
16	5	18	-		
18	6.5	24	_		
20	8	24	_		

NOTES:

THRUST BLOCKS SHALL BE REQUIRED FOR ALL PUMPLINES

ALL VALUES SHOWN ARE FOR A HYDROSTATIC PRESSURE OF 50 PSI AND A SOIL RESISTANCE OF 2000 PSF. TID WILL CONSIDER REDUCTION OF THRUST BLOCK SURFACE AREA UPON SUBMITTAL OF APPROVED SOIL RESISTANT TEST RESULTS GREATER THAN 2000 PSF

ALL THRUST BLOCKS SHALL BE POURED AGAINST UNDISTURBED SOIL

				IRRI	IGATION DISTRICT					ICT	IRRIGATION Construction standards			
	STDS.	COM. AF	PROVAL			2	BJ	LE			THR	UST E	BLOCK	S
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